Exhibit A

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### BEFORE THE ENVIRONMENTAL APPEALS BOARD UNITED STATES ENVIRONMENTAL PROTECTION AGENCY WASHINGTON, D.C.

In re: Dry Creek Rancheria

NPDES Appeal No.

NPDES Permit No CA 0005241

### **EXHIBITS TO PETITION FOR REVIEW**

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### LIST OF EXHIBITS

- A. Permit Application and Engineering Report (Feb. 17, 2005).
- B. Letter from Region to Permittee requesting additional information (May 27, 2005).
- C. Letter from Permittee to Region with additional application information (June 30, 2005).
- D. Letter from Petitioners to Region regarding the Permit Application (March 21, 2006).
- E. Proposed Permit (June 29, 2006).
- F. Statement of Basis for Proposed Permit (June 29, 2006).
- G. Letter from Congressman Mike Thompson to Region regarding the proposed permit (Sept. 18, 2006).
- H. Letter from AVA to Region regarding the proposed permit (Sept. 27, 2006).
- I. Letter from Petitioners to Region regarding the proposed permit (Sept. 29, 2006).
- J. Letter from Regional Board to Region regarding the proposed permit (Oct. 2, 2006).
- K. Memo from Ginette Chapman, EPA Region 9 Office of Regional Counsel to record re conference call with the Office of Senator Barbara Boxer (Oct. 6, 2006).
- L. Letter from Permittee to Petitioners with first water balance chart (April 17, 2007).
- M. Second draft of water balance chart (sent from Region to Petitioners on April 20, 2007).
- N. Technical memorandum and third draft of water balance chart (sent from Region to Petitioners on April 25, 2007).
- O. Letter from Petitioners to Region re water balance information (April 27, 2007).

Exhibits to Petition for Review of NPDES Permit No CA 0005241

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- P. E-mail from Region to Petitioners re final water balance technical memorandum (April 30, 2007).
- Q. California Department of Water Resources, A Guide to Estimating Irrigation Water Needs of Landscape Plantings in California, pages 1-22 (Aug. 2000)
- R. Final Permit (April 30, 2007).

S. Final Statement of Basis (April 30, 2007).

- T. Responses to Comments Document (April 30, 2007).
- U. Final technical memorandum and water balance chart (included as Appendix 3 to Responses to Comments Document) (April 30, 2007).

Exhibits to Petition for Review of NPDES Permit No CA 0005241



February 17, 2005

Susan Saucerman (WTR-5) CWA Standards and Permits Office USEPA Region 9 75 Hawthorne Street San Francisco, CA 94105

Dear Ms. Saucerman:

Subject: NPDES Permit Application and Engineering Report for the Dry Creek WWTP Dry Creek Band of Pomo Indians

The Dry Creek Band of Pomo Indians has constructed a wastewater treatment plant near the City of Geyserville, CA in Sonoma County, CA. This plant provides tertiary treatment of sewage generated by existing and future Tribal facilities, including the existing casino.

Dry Creek Rancheria BAND OF POMO INDIANS

A comprehensive program for reuse and disposal of treated wastewater has been developed, which includes discharge to surface water as one component. This permit application is submitted to support the NPDES effluent discharge permit to allow the discharge of a portion of the treated wastewater from the Dry Creek WWTP to surface waters on Tribal lands. This permit application package consists of the

- 1. USEPA Form 1: General Information, Consolidated Permits Program
- 2. USEPA Form 2A: Basic Application Parts A, B, and C
- 3. Engineering Report describing the Project, treatment process, design parameters, and anticipated discharge permit limitations

Additional work to support this planned surface water discharge includes a biological evaluation of the impacts of this discharge to aquatic habitat, and a technical memorandum about the rapid bioassessment in drainages P-1 and A-1. These documents are included as attachments to this application.

Should you have any questions about this NPDES application or the Engineering Report, please do not hesitate to contact me at (707) 473-2182.

Sincerely yours,

Don hul

Thomas Keegan. Director of Environmental Protection

Enclosures (3)

## **USEPA Application Form 1:**

General Information, Consolidated Permits Program

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# **USEPA Form 2A:**

Basic Application – Parts A, B, and C

FACILITY NAME AND PERMIT NUMBER: Dry Creek Rancheria WWTP

Form Approved 1/14/99 OMB Number 2040-0086 -

#### FORM 2A

NPDES

## NPDES FORM 2A APPLICATION OVERVIEW

### **APPLICATION OVERVIEW**

Form 2A has been developed in a modular format and consists of a "Basic Application Information" packet and a "Supplemental Application Information" packet. The Basic Application Information packet is divided into two parts. All applicants must complete Parts A and C. Applicants with a design flow greater than or equal to 0.1 mgd must also complete Part B. Some applicants must also complete the Supplemental Application Information packet. The following items explain which parts of Form 2A you must complete.

### BASIC APPLICATION INFORMATION:

- Basic Application Information for all Applicants. All applicants must complete questions A.1 through A.8. A treatment works that discharges effluent to surface waters of the United States must also answer questions A.9 through A.12.
- Additional Application Information for Applicants with a Design Flow ≥ 0.1 mgd. All treatment works that have design Β. flows greater than or equal to 0.1 million gallons per day must complete questions B.1 through B.6.
- C. Certification. All applicants must complete Part C (Certification).

## SUPPLEMENTAL APPLICATION INFORMATION:

- Expanded Effluent Testing Data. A treatment works that discharges effluent to surface waters of the United States and D meets one or more of the following criteria must complete Part D (Expanded Effluent Testing Data):
  - 1. Has a design flow rate greater than or equal to 1 mgd,
  - 2. Is required to have a pretreatment program (or has one in place), or
  - 3. Is otherwise required by the permitting authority to provide the information.
- Toxicity Testing Data. A treatment works that meets one or more of the following criteria must complete Part E (Toxicity E.
  - 1. Has a design flow rate greater than or equal to 1 mgd,
  - 2. Is required to have a pretreatment program (or has one in place), or
  - 3. Is otherwise required by the permitting authority to submit results of toxicity testing.
- Industrial User Discharges and RCRA/CERCLA Wastes. A treatment works that accepts process wastewater from any F. significant industrial users (SIUs) or receives RCRA or CERCLA wastes must complete Part F (Industrial User Discharges and RCRA/CERCLA Wastes). SIUs are defined as:
  - 1. All industrial users subject to Categorical Pretreatment Standards under 40 Code of Federal Regulations (CFR) 403.6 and 40 CFR Chapter I, Subchapter N (see instructions); and
  - 2. Any other industrial user that:
    - Discharges an average of 25,000 gallons per day or more of process wastewater to the treatment works (with certain а.

    - b. Contributes a process wastestream that makes up 5 percent or more of the average dry weather hydraulic or organic
    - .c. Is designated as an SIU by the control authority.
- Combined Sewer Systems. A treatment works that has a combined sewer system must complete Part G (Combined Sewer G

# ALL APPLICANTS MUST COMPLETE PART C (CERTIFICATION)

FACILITY NAME AND PERMIT NUMBER:

Dry Creek Rancheria WWTP

Form Approved 1/14/99 OMB Number 2040-0086

	PLICATION INFORMATION FOR AL		
ll treatment works mu	st complete questions A.1 through A.8 o	f this Basic Application Information	
1. Facility Information	on.		packet
Facility named	Dry Creek Rancheria - Wastewater Recla	mation Facility	
Mailing Address	P.O. Box 607		
	Geyserville, CA 95441		
Contact person	Tom Keegan		
Title	Environmental Director	e e e	
Telephone number	(707) 473-2178		
Facility Address	3250 Highway 128 East		
(not P.O. Box)	Dry Creek Rancheria, CA 95441		
Applicant Informat	on. If the applicant is different from the abov	/e, provide the following	
Applicant name	Same as above		
Mailing Address			· · ·
Contact person			
Title			
Telephone number	·		
Is the applicant the (	owner or operator (or both) of the treatme	ent works	
X owner	X operator		
Indicate whether corre	spondence regarding this permit should be d	irected to the facility or the applicant.	
Evicting Course			
(include state-issued p	tal Permits. Provide the permit number of a ermits).	ny existing environmental permits that h	ave been issued to the treatment works
NPDES	N/A		
UIC	N/A	PSD	N/A
RCRA	N/A	Other	<u>N/A</u>
	ormation. Provide information on municipali	ties and areas served by the facility.	N/A
Collection System Inf entity and, if known pro	VICE information on the t	iom (combined	Jvide the name and population of each
Collection System Inf entity and, if known, pro Name	Population on the type of collection syst	(combined vs. separate) and its owr	nership (municipal, private, etc.).
Collection System Inf entity and, if known, pro Name Plant Operations Divisio	Population Served	Type of Collection System	nership (municipal, private, etc.). <b>Ownership</b>
Collection System Inf entity and, if known, pro Name Plant Operations Divisio	vide information on the type of collection syst  Population Served  Predominantly transient  Population	Type of Collection System Separate	nership (municipal, private, etc.). <b>Ownership</b> Tribal Government

€PA Form 3510-2A (Rev. 1-99). Replaces EPA forms 7550-6 & 7550-22.

	DELLY NAME AND PERMIT NUMBER:			,	Form	Approved 1/14/99	
		· · · · · · · · · · · · · · · · · · ·			OWB	Number 2040-0086	5
5.	Indian Country.						
	a. Is the treatment works located in Indian C	Country?				•	
	X Yes N	No.			•		
	b. Does the treatment works discharge to a through) Indian Country?	receiving water that is either in	Indian Country or that	t is upstrea	n from (and even	tually flows	
	X YesN	ło					
•	Flow. Indicate the design flow rate of the trea daily flow rate and maximum daily flow rate for month of "this year" occurring no more than th	tment plant (i.e., the wastewate r each of the last three years. If ree months prior to this applica	er flow rate that the pla Each year's data must ation submittal.	int was buil be based c	t to handle). Also on a 12-month tim	provide the avera e period with the 1	ge I 2th
i	a. Design flow rate0.15 mgd	на се	· · · ·	•	•	· · · ·	
	· · · · · · · · · · · · · · · · ·	Two Years Ado (2003)	Last Year (2004)		This Vers (200	E mania at 15	
I	b. Annual average daily flow rate	0.015	<u></u> (2004)	·	<u>inis rear</u> (200	o projected)	•
. (	c. Maximum daily flow rate	0.032	0.047		0.04	mgd `	
	0-11		0.047		0.06	mgd	
(	Collection System. Indicate the type(s) of co contribution (by miles) of each.	pllection system(s) used by the	treatment plant. Cheo	ck all that a	opły. Also estima	te the percent	
-	X Separate sanitary sewer	•	•	•	100		
-	Combined storm and sanitary sewer	· · ·		-	100	%	
r	Discharges and Other Discout M. d	•	. *	-		%	
L	Discharges and Other Disposal Methods.						
а	a. Does the treatment works discharge efflue	nt to waters of the U.S.?		х	Yes	No	
	If yes, list how many of each of the followin	g types of discharge points the	treatment works uses	<u>_</u>		NO .	
	i. Discharges of treated effluent				2 (proje	cted) 0 (current)	
	ii. Discharges of untreated or partially tre				2 I I I I I I I I I I I I I I I I I I I	Ceul o Clinenn	
	See of antiouted of partially de	ated effluent			2. (proje	0	
	iii. Combined sewer overflow points	ated effluent				0	
	iii. Combined sewer overflow points iv. Constructed emergency overflows (price	ated effluent				0 0 0	
	iii. Combined sewer overflow points iv. Constructed emergency overflows (pric v. Other	ated effluent or to the headworks)	· · · · · · · · · · · · · · · · · · ·			0 0 0	
	iii. Combined sewer overflow points iv. Constructed emergency overflows (pric v. Cther	ated effluent	· · · · · · · · · · · · · · · · · · ·			0 0 0 0 None	
b.	<ul> <li>iii. Combined sewer overflow points</li> <li>iv. Constructed emergency overflows (pric</li> <li>v. Other</li> <li>b. Does the treatment works discharge effluer that do not have outlets for discharge to wa</li> </ul>	ated effluent or to the headworks) 	rface impoundments	• • •	Yes	0 0 0 None	
b.	<ul> <li>iii. Combined sewer overflow points</li> <li>iv. Constructed emergency overflows (price</li> <li>v. Other</li> <li>b. Does the treatment works discharge effluer that do not have outlets for discharge to wall figures, provide the following <u>for each surface</u></li> </ul>	ated effluent or to the headworks) 	rface impoundments	- - -	  Yes	0 0 0 None X No	
b.	<ul> <li>iii. Combined sewer overflow points</li> <li>iv. Constructed emergency overflows (price</li> <li>v. Other</li> <li>b. Does the treatment works discharge effluer that do not have outlets for discharge to wa</li> <li>If yes, provide the following <u>for each surface</u></li> <li>Location: N/A</li> </ul>	ated effluent or to the headworks) 	rface impoundments		Yes	0 0 0 None X No	
	<ul> <li>iii. Combined sewer overflow points</li> <li>iv. Constructed emergency overflows (price v. Gther</li></ul>	ated effluent or to the headworks) 	rface impoundments		Yes	0 0 0 None X No	
	iii. Combined sewer overflow points iv. Constructed emergency overflows (pric v. Cther b. Does the treatment works discharge effluer that do not have outlets for discharge to wa If yes, provide the following <u>for each surface</u> Location: N/A Annual average daily volume discharged to Is discharge continuous or	ated effluent or to the headworks) nt to basins, ponds, or other suiters of the U.S.? impoundment: surface impoundment(s) intermittent?	rface impoundments	N/A	Yes r	0 0 0 None X No	
	iii. Combined sewer overflow points iv. Constructed emergency overflows (price v. Other b. Does the treatment works discharge effluer that do not have outlets for discharge to wall figures, provide the following <u>for each surface</u> Location: N/A Annual average daily volume discharged to Is discharge continuous or c. Does the treatment works land-apply treated	ated effluent or to the headworks) nt to basins, ponds, or other sui- ters of the U.S.? <u>e impoundment</u> : surface impoundment(s) intermittent? d wastewater?	rface impoundments	N/A	Yes r	0 0 0 None X No	
	iii. Combined sewer overflow points iv. Constructed emergency overflows (price v. Gther b. Does the treatment works discharge effluer that do not have outlets for discharge to was if yes, provide the following <u>for each surface</u> Location: N/A Annual average daily volume discharged to is discharge continuous or c. Does the treatment works land-apply treated if yes, provide the following <u>for each land apply</u> the followin	ated effluent or to the headworks) nt to basins, ponds, or other suiters of the U.S.? e impoundment: surface impoundment(s) intermittent? d wastewater? iplication site:	rface impoundments	N/AX	Yes	0 0 0 None X No	
с.	iii. Combined sewer overflow points iv. Constructed emergency overflows (price v. Other b. Does the treatment works discharge effluer that do not have outlets for discharge to wal if yes, provide the following <u>for each surface</u> Location: N/A Annual average daily volume discharged to is discharge	ated effluent or to the headworks) nt to basins, ponds, or other suiters of the U.S.? impoundment: surface impoundment(s) intermittent? d wastewater? iplication site: ray-field Application	rface impoundments	N/A X	Yes	0 0 0 None X No ngd No	
	iii. Combined sewer overflow points iv. Constructed emergency overflows (price v. Other b. Does the treatment works discharge effluer that do not have outlets for discharge to wa If yes, provide the following for each surface Location: N/A Annual average daily volume discharged to Is discharge continuous or C. Does the treatment works land-apply treated If yes, provide the following for each land ap Location: Landscape Irrigation and Spr Number of acres: Plans for up to 16 a	ated effluent or to the headworks)  nt to basins, ponds, or other suiters of the U.S.?  impoundment:  surface impoundment(s)  intermittent?  d wastewater?  plication site: ray-field Application cres, total	rface impoundments	N/A X	Yes	0 0 0 None X No	
с.	<ul> <li>iii. Combined sewer overflow points</li> <li>iv. Constructed emergency overflows (price</li> <li>v. Cther</li> <li>b. Does the treatment works discharge effluer that do not have outlets for discharge to wan if yes, provide the following for each surface</li> <li>Location: N/A</li> <li>Annual average daily volume discharged to is discharge continuous or</li> <li>c. Does the treatment works land-apply treated if yes, provide the following for each land ap Location: Landscape Irrigation and Spr Number of acres: Plans for up to 16 a Annual average daily volume applied to site:</li> </ul>	ated effluent or to the headworks)  nt to basins, ponds, or other suiters of the U.S.?  impoundment:  surface impoundment(s)  intermittent?  d wastewater?  plication site: ray-field Application cres, total  0.03	rface impoundments	N/A X	Yes	0 0 0 None X No ngd No	
	<ul> <li>iii. Combined sewer overflow points</li> <li>iv. Constructed emergency overflows (price</li> <li>v. Other</li> <li>b. Does the treatment works discharge effluer that do not have outlets for discharge to wan if yes, provide the following for each surface</li> <li>Location: N/A</li> <li>Annual average daily volume discharged to is discharge continuous or</li> <li>c. Does the treatment works land-apply treated if yes, provide the following for each land application continuo</li> </ul>	ated effluent or to the headworks)  nt to basins, ponds, or other suiters of the U.S.?  impoundment:  surface impoundment(s)  intermittent?  wastewater?  plication site: ray-field Application  cres, total  0.03  pus or intermittee	rface impoundments	N/A X	Yes	0 0 0 None X No ngd No	
c.	iii. Combined sewer overflow points iv. Constructed emergency overflows (price v. Other b. Does the treatment works discharge effluer that do not have outlets for discharge to wal if yes, provide the following for each surface Location: N/A Annual average daily volume discharged to is discharge continuous or c. Does the treatment works land-apply treated if yes, provide the following for each land application: Landscape Irrigation and Spri Number of acres: Plans for up to 16 a Annual average daily volume applied to site: Is land application continuo	ated effluent or to the headworks)  Int to basins, ponds, or other suiters of the U.S.?  a impoundment:  surface impoundment(s)	rface impoundments	N/A X	Yes	0 0 0 None X No ngd No	
b c.	iii. Combined sewer overflow points iv. Constructed emergency overflows (price v. Other b. Does the treatment works discharge effluer that do not have outlets for discharge to wall figs, provide the following for each surface Location: N/A Annual average daily volume discharged to is discharge continuous or c. Does the treatment works land-apply treated if yes, provide the following for each land apple. Location: Landscape Irrigation and Springe. Number of acres: Plans for up to 16 a Annual average daily volume applied to site: Is land application continuo Does the treatment works discharge or transtreatment works?	ated effluent  pr to the headworks)  nt to basins, ponds, or other suiters of the U.S.?  impoundment:  surface impoundment(s)  intermittent?  d wastewater?  plication site: ray-field Application cres, total  0.03  pus or intermitte  port treated or untreated wastem	rface impoundments	N/A X	Yes	0 0 0 None X No ngd No	

Dry (	Creek Rancheria WWT	P	•		•	Form Appro OMB Numb	oved 1/14/99 ber 2040-008
	If yes, describe the me (e.g., tank truck, pipe).	an(s) by which the wastewate	er from the treatmen	t works is discharged	or transported to	the other treatme	ent works
	· · · · · · · · · · · · · · · · · · ·				•.		
•	If transport is by a party	other than the applicant pro	wide				
	Transporter name:	and approxim, pre	JAIGE.				
	Mailing Address:	· ·	· · ·			·····	
		and a second second				·	_ <u></u>
	•			<u> </u>		<u> </u>	
(	Contact person:		· · · · · · · · · · · · ·				
7	Title:		· · · · · · · · · · · · · · · · · · ·	•			<u>.</u>
٦	Telephone number:				•	· · · · · · · · · · · · · · · · · · ·	•
·				-			
E	or each treatment work	s that receives this discharge	e, provide the followi	ng:			
	•					1990 - E	
Ν	lame:				<u>.</u>		
N	failing Address:			•			
С	ontact person:						
T	itle:						<del></del>
Т	elephone number:						
lf	known, provide the NPI	DES permit number of the tre					<u> </u>
Pi	rovide the average daily	flow rate from the trootment	aument works that re	eceives this discharge		·	
		weth tate in on the treatment	works into the receiv	ing facility.			mgd
Do A.	bes the treatment works 8.a through A.8.d above	discharge or dispose of its we e (e.g., underground percolation	vastewater in a manr ion, well injection)?	ner not included in	Ya	• • •	
lfy	yes, provide the followin	g for each disposal method:			I e.	°	No
De	escription of method (ind N/	cluding location and size of si	ite(s) if applicable):	•		· , .	
An	inual daily volume dispo	sed of by this method:					
ls i	disposal through this m	ethod				1	

E A.	CILIT		A MIN' NEW YORANY	611 ) B 4 70 P m
- ~		INAME	AND PERMIT	

Dry Creek Rancheria WWTP

Form Approved 1/14/99 OMB Number 2040-0086

#### WASTEWATER DISCHARGES:

If you answered "yes" to question A.8.a, complete questions A.9 through A.12 once for each outfall (including bypass points) through which effluent is discharged. Do not include information on combined sewer overflows in this section. If you answered "no" to question A.8.a, go to Part B, "Additional Application Information for Applicants with a Design Flow Greater than or Equal to 0.1 mgd."

A.9.	D	escription of Outfall.		-	<u>-</u>				<u> </u>
	а.	Outfall number	P1-1	•					
	h	Location	Des Creats Des stants	• •	• . •	· · .		•	
ĺ		coulon	(City or town, if applicable)	•	(Zin	05441			
			Sonoma (County)		CA				
			38° 42' 06" N		(Stat 122 <sup>°</sup> 51' 31" W	te)			
		_ ·	(Latitude)		(Lon	gitude)			
	c.	Distance from shore (if	applicable)	N/A	ft.				
	d.	Depth below surface (if	applicable)	N/A	ft.				
	e.	Average daily flow rate	(2005 Projected)	OCT 1 - MAY 14: 0.039 MAY 15 - SEP 30: 0.000	mgd		· · ·		
	f.	Does this outfall have ei	ther an intermittent or a pariadia	•	· .	-			
		discharge?						• • •	
		If ves, provide the follow	ing information.	Yes	<u> </u>	No (	go to A.9.g.)		
		. yes, provide are follow	ing information.				•		2 C - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2
		Number of times per yea	ar discharge occurs:				÷		-
		Average duration of eacl	discharge:					•	
		Average flow per dischar	ge:			mad			
		Months in which dischar	ge occurs:			mga .			
	•		· · ·						
	g.	Is outfall equipped with a	diffuser?	Yes	<u>x</u>	No		•.	
			÷ .	· .	×				•
A.10.	Des	scription of Receiving W	laters.					•	
	a.	Name of receiving water	Linnamod soosonat a	and (D4) T is a second				•	
		er recenting mater	Stanled seasonal C	reek (P1) - Indutary to the	Russian River	·	·		
ì	b.	Name of watershed (if kn	own) Ri	ussian River				-	
					······································		······································		
		United States Soil Conse	rvation Service 14-digit watershed	code (if known):					
	3.	Name of State Managem	ent/River Basin (if known):	Russian River					
		United States Geological	Survey 8-digit bydrologic catalogi	an unit onde (61					
				ig unit code (if known):		10110			
c	<b>1</b> .	Critical low flow of receiving	ng stream (if applicable):					•	
		acute 0	cfs	chronic 0	cfs				
e	<u>.</u>	Total hardness of receivin	g stream at critical low flow (if app	blicable): N/A	mg/l of CaCO	3		,	
						5			
		;							
				• • •					
		· · · ·	· · · ·		· ·		•		

EPA Form 3510-2A (Rev. 1-99). Replaces EPA forms 7550-6 & 7550-22.

•						
• •						· .
en e	et e se s	a da anti-			1	
			•			
ILITY NAME AND PERMIT	NUMBER:		•			
Dry Creek Rancheria WWTP				* •	Form A OMB N	pproved 1. umber 20
WASTEWATER DISCHARG	ES	<u>_</u>				
	-0.	· · · · · · ·	•	·		
Description of Outfall.						
Description of Outfall						
a. Outfall number	A1 4		•			
		· · · · · · · · · · · · · · · · · · ·				
b. Location	Day Creat D		•	• • •	· • •	•
	Dry Creek Rancheria	1				
	City or town, if applicable)			95441		• .
	City or town, if applicable) Sonoma (County)			95441 (Zip Code) CA		• .
	Creek Kancheria (City or town, if applicable) Sonoma (County) 38°42' 19" N		1720 5 11 25	95441 (Zip Code) CA (State)		• .
	City or town, if applicable) Sonoma (County) 38°42' 19" N (Latitude)		122°51' 35	95441 (Zip Code) CA " N		• . 
c. Distance from shore (if a	Creek Kancheria (City or town, if applicable) Sonoma (County) 38°42' 19" N (Latitude) Oplicable)		122°51'35	95441 (Zip Code) CA "N (State) (Longitude)		• . 
c. Distance from shore (if an	Creek Kancheria (City or town, if applicable) Sonoma (County) 38°42' 19" N (Latitude) Oplicable)	N/A	122°51'35ft.	95441 (Zip Code) CA (State) " N (Longitude)		
c. Distance from shore (if ap d. Depth below surface (if ap	Creek Rancheria (City or town, if applicable) Sonoma (County) 38°42' 19" N (Latitude) oplicable)	N/A	122°51'35 ft.	95441 (Zip Code) CA " N (State) (Longitude)		
c. Distance from shore (if ap d. Depth below surface (if ap e. Average daily flow rate ()	Creek Rancheria (City or town, if applicable) Sonoma (County) 38° 42' 19" N (Latitude) Oplicable) 2005 Projected)	N/A	122°51'35 ft. ft.	<u>95441</u> (Zip Code) CA " N (State) (Longitude)		
<ul> <li>c. Distance from shore (if and</li> <li>d. Depth below surface (if and</li> <li>e. Average daily flow rate (</li> </ul>	Creek Rancheria (City or town, if applicable) Sonoma (County) 38° 42' 19" N (Latitude) oplicable) 2005 Projected)	<u>N/A</u> <u>N/A</u> 0.001	122°51'35 ft. ft. mgd	95441 (Zip Code) CA "N <sup>(State)</sup> (Longitude)	<u>.</u>	
<ul> <li>c. Distance from shore (if ar</li> <li>d. Depth below surface (if ar</li> <li>e. Average daily flow rate (</li> <li>Does this outfall have either</li> </ul>	Dry Creek Rancheria         (City or town, if applicable)         Sonoma         (County)         38°42' 19" N         (Latitude)         Oplicable)         2005 Projected)	N/A N/A 0.001	122°51'35 ft. ft. mgd	95441 (Zip Code) CA " N (State) (Longitude)		
<ul> <li>c. Distance from shore (if and a shore) (if and a shore) (if and a shore) (if and a shore) (if a sho</li></ul>	Dry Creek Rancheria         (City or town, if applicable)         Sonoma         (County)         38° 42' 19" N         (Latitude)         opplicable)         2005 Projected)         er an intermittent or a periodic	N/A N/A 0.001	122°51'35 ft. ft. mgd	95441 (Zip Code) CA "N <sup>(State)</sup> (Longitude)		
<ul> <li>c. Distance from shore (if and a shore) (if a</li></ul>	Creek Rancheria (City or town, if applicable) Sonoma (County) 38° 42' 19" N (Latitude) oplicable) 2005 Projected) er an intermittent or a periodic	<u>N/A</u> <u>N/A</u> 0.001 Yes	122°51'35 ft. ft. ft. mgd	95441 (Zip Code) CA " N (State) (Longitude)		

If yes, provide the following information:	YesX No	(go to A.9.g.)
Number of times per year discharge occurs:		
Average duration of each discharge:		•

Average flow per discharge:	
Months in which discharge occurs:	

# g. Is outfall equipped with a diffuser?

## A.10. Description of Receiving Waters.

a. Name of receiving water	Linear 1		•	•
water	 Unitamed seasonal creek (A1) - isolated interview			
		hutanut	in Due	- i
		outary t	o Rus	sian River

b	Name of watershed (if known)	Russian River
	United States Soil Conservation Service 14-digit	watershed code (if known):

с. <sup>.</sup>	Name of State Management/River Basin (if known)	•
		Russian River

United States Geological Survey 8-digit hydrologic cataloging unit code (if known): 18010110

d. Critical low flow of receiving stream (if applicable): acute \_\_\_\_\_ cfs

chronic \_ e. Total hardness of receiving stream at critical low flow (if applicable): 0 \_\_ cfs N/A

\_\_ mg/l of CaCO<sub>3</sub>

Form Approved 1/14/99 OMB Number 2040-0086

mgd

No

Dry Creek Rand	heria WWTP		••••••••••••••••••••••••••••••••••••••			. •		OMB Number	2040-0086
A.11. Description	of Treatment.	•	1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -		l <u>-</u>			· · · · · ·	
a. What lev	els of treatment	are provided? (	Check all that a	pply.				•	
X	Primary		X Sec	condary	·			•	ц
X	Advanced		Oth	er. Describe:	•	. •	• .		
b. Indicate t	ne following ren	noval rates (as a	applicable):			•		······	
Design B	OD, removal <u>or</u>	Design CBOD	removal			, 99			
Design S	S removal		•••••	· · ·	· · ·		/0	• . • •	
Desian P	removal				· · · · · · · · · · · · · · · · · · ·	99	%		
- Docian M		an a	• • • •	•	· ·,	73	%	•	
Design N	removal	•				84	%		
Other _	lurbidity			•		< 1 N	TU%		
c. What type	of disinfection	is used for the	effluent from th	is outfall? If disir	nfection varies	by season, p	lease describe.	•	
Ultravio	et (UV) disinfec	tion and Chlorir	nation	· · · · · · · · · · · · · · · · · · ·			•	•	
If disinfect	ion is by chlorir	nation, is dechlo	rination used fo	or this outfall?		×	Yes	Nic	
d. Does the t	reatment plant i	have post aerati	on?					140	
discharged. collected thrc 40 CFR Part 1 minimum, effi	Provide the ind Do not include ugh analysis of 36 and other a uent testing d DATA	licated effluent information o conducted usin appropriate QA ata must be ba	t testing requi n combined se ng 40 CFR Par VQC requirem used on at leas LE - FACILITY	red by the perm ewer overflows t 136 methods. ents for standa st three sample	of the US mus nitting author in this sectio In addition, ord methods f s and must be DECEMBER 20	it provide e ity <u>for each</u> n. All infon this data m or analytes e no more t	fluent testing da outfall through y nation reported r ust comply with ( not addressed b han four and one	ta for the fol which effluer nust be base QA/QC requi QA/QC requi y 40 CFR Pa -half years	lowing <u>nt is</u> ed on data rements of rt 136. At a apart.
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BASIC APPLICATION INFORMATION PART B. ADDITIONAL APPLICATION INFORMATION FOR APPLICANTS WITH A DESIGN FLOW GREATER THAN OF EQUAL TO 0.1 MCD (100,000 grinds per day). All applicants with a design flow rate 2 0.1 ngd must answer questions B.1 through B.6. All others go to Part C (Cartification). B.1. Inflow and Infiltration. Estimate the average number of gallons per day bat flow into the teatment works from inflow and/or infiltration	Dr	ry Creek Rancheria WWTP			Form Approved 1/14/99 OMB Number 2040-0086
PART B.       ADDITIONAL APPLICATION INFORMATION FOR APPLICANTS WITH A DESIGN FLOW GREATER THAN OF EQUAL TO 0.1 MGD (100,000 gallons per day).         VI applicants with a design flow rate 2.0.1 mgd must answer questions B.1 through B.6. All others go to Part C (Certification).         3.1.       Inflow and Infitration. Estimate the average number of gallons per day that flow into the treatment works from inflow and/or infitration.         3.0.00       [225 max]       .gdd         Briefly explain any steps underway or planned to minimize inflow and infitration.	BASI	C APPLICATION INFORMATIO	N		
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Usepticians with a design flow rate ≥ 0.1 mgd must enswer questions 8.1 through 8.6. At others go to Part C (Certification).         3.100       1/1/47 max.)god         3.11       Inflow and Infiltration. Estimate the average number of gallons per day that flow into the treatment works from inflow and/or infiltration.         3.000       1/27 max.)god         Briefly explain any steps underway or planned to minimize inflow and infitration.		EQUAL TO 0.1 MGD (100,000 gallor	MATION FOR APPLICA is per day).	NTS WITH A DESIGN FLOW	GREATER THAN OF
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3.000	3.1. Inf	low and Infiltration. Estimate the average numb	er of gallons per day that flow	inte the t	)
	3,0	000 (2% max.)gpd	on any marnow	into the treatment works from inflow	and/or infiltration.
<ul> <li>2. Topographic Map. Attach to this application a topographic map of the area extending at least one mile beyond facility property boundanes. Thi map must show the outline of the facility and the following information. (You may submit more than one map if one map does not show the entire area.)</li> <li>a. The area surrounding the treatment plant, including all unit processes.</li> <li>b. The major pipes or other structures through which wastewater enters the treatment works and the pipes or other structures through which mastewater enters the treatment by an underground.</li> <li>c. Each well where wastewater from the treatment plant. Include outfalls from bypass piping. If applicable.</li> <li>c. Each well where wastewater from the treatment plant. Include outfalls from bypass piping. If applicable.</li> <li>d. Any areas where the sewage sludge produced by the treatment works is stored, treated, or disposed.</li> <li>f. If the treatment works receives waste that is classified as hazardous under the Resource Conservation and Recovery Act (RCRA) by truck, ra or special pipe, show on the map where that hazardous waste enters the treatment towks receives waste has that are avained waste enters the treatment plant, including all treated.</li> <li>Process Flow Diagram or Schematic. Provide a diagram showing the processes of the treatment plant, including all toppass piping and all backup or redundancy in the system. Also provide a water belance showing all treatment units, including all toppass piping and all backup.</li> <li>Operation/Maintenance Performed by Contractor(s).</li> <li>Are argo operational or maintenance aspects (related to wastewater treatment and effluent quality) of the treatment works the responsibilities (attach additional pages in the exact works and the set works is of each contractor's responsibilities (attach additional pages in the exact works, the phone number. And status of each contractor and describe the contractor's responsibilities (atta</li></ul>	Brie	efly explain any steps underway or planned to mini	mize inflow and infiltration.		
2. Topographic Map. Attach to this application a topographic map of the area extending at least one mile beyond facility property boundanes. Thi map must show the outline of the facility and the following information. (You may submit more than one map if one map does not show the entire area surrounding the treatment plant, including all unit processes. 3. The area surrounding the treatment plant, including all unit processes. 4. The major pipes or other structures through which wastewater enters the treatment works and the pipes or other structures through which treated wastewater is discharged from the treatment plant. Include outlists from bypass piping, if applicable. C. Each well where wastewater from the treatment plant is injected underground. 4. Wells, springs, other surface water bodges, and dinking water wells that are: 1) within 1/4 mile of the property boundaries of the treatment works, and 2) listed in public record or otherwise known to the applicant. e. Any areas where the sewage sludge produced by the treatment works is stored, treated, or disposed. 1. If the treatment work spreakes waste that is classified as hazardous under the Resource Conservation and Recovery Act (RCRA) by truck, ra or special pipe, show on the map where the taxing at water balance showing all treatment torks and where it is treated, stored, and/or disposed. Process Flow Diagram or Schematic. Provide a diagram showing the processes of the treatment plant, including all toppass piping and all backur devilonmation. The water balance must have dainy average flow rates at influent quality) of the treatment works the responsibility for a linearment. Enclude a bine fraatmence aspects (related to wastewater treatment and effluent quality) of the treatment works the responsibility of a linearment units. Include a bine fraatmence aspects (related to wastewater treatment and effluent quality) of the treatment works the responsibility of a lineassexity. Neme:	·				
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<ul> <li>b. The major pipes or other structures through which wastewater enters the treatment works and the pipes or other structures through which treated wastewater is discharged from the treatment plant. Include outfalls from bypass piping, if applicable.</li> <li>c. Each well where wastewater from the treatment plant is injected underground.</li> <li>d. Wells, springs, other structures where holes, and drinking water wells that are: 1) within 1/4 mile of the property boundaries of the treatment works, and 2) listed in public record or otherwise known to the applicant.</li> <li>e. Any areas where the sewage sludge produced by the treatment works is stored, treated, or disposed.</li> <li>f. If the treatment works receives waste that is classified as hazardous under the Resource Conservation and Recovery Act (RCRA) by truck, ra or special pipe, show on the map where that hazardous waste enters the treatment works and where it is treated, stored, and/or disposed.</li> <li>Process Flow Diagram or Schematic. Provide a diagram showing the processes of the treatment plant, including disinfection (e.g. chlorination). The water balance must show daily average flow values at influent and discharge points and approximate daily flow rates between treatment units. Includie a binef narrative description of the diagram.</li> <li>Operation/Maintenance Performed by Contractor(s).</li> <li>Are any operational or maintenance aspects (related to wastewater treatment and effluent quality) of the treatment works the responsibility of a contractor <u>yes is the name</u>, address, telephone number, and status of each contractor and describe the contractor's responsibilities (attach additional pages the name: address, telephone number, and status of each contractor and describe the contractor's responsibilities (attach additional pages incompleted plans for imporvements that will affect the wastewater treatment, effluent quality, or design capacity of the treatment works. If the reatment works has several different implementation. Provid</li></ul>	a <sup>-</sup>	The area surrounding the treatment plant, including	all unit processor		
<ul> <li>c. Each well where wastewater from the treatment plant is injected underground.</li> <li>d. Wells, springs, other surface water bodies, and drinking water wells that are: 1) within 1/4 mile of the property boundaries of the treatment works, and 2) listed in public record or otherwise known to the applicant.</li> <li>e. Any areas where the sewage sludge produced by the treatment works is stored, treated, or disposed.</li> <li>f. If the treatment works receives waste that is classified as hazardous under the Resource Conservation and Recovery Act (RCRA) by truck, ra or special pipe, show on the map where that hazardous waste enters the treatment works and where it is treated, stored, and/or disposed.</li> <li>Process Flow Diagram or Schematic. Provide a diagram showing the processes of the treatment plant, including disinfection (e.g., choinnation and treatment units. include a brief narrative description of the diagram.</li> <li>Operation/Maintenance Performed by Contractor(s).</li> <li>Are any operational or maintenance aspects (related to wastewater treatment and effluent quality) of the treatment works the responsibility of a increasmy. Improvements and Schedules of Implementation. Provide information on any uncompleted implementation schedule or incompleted plans for improvements that will affect the wastewater treatment, effluent quality, or design capacity of the treatment works. If the eatment works has several different implementation sche</li></ul>	b. 1 t	The major pipes or other structures through which treated wastewater is discharged from the treatment	wastewater enters the treatment	ent works and the pipes or other stru	ictures through which
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c If the answer to B	.5.b is "Yes," briefly d	describe, includi	ng new maxir	num dáily inflow	v rate (if one lie - 1)			
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• •						<u> </u>	•	
<ul> <li>d. Provide dates imported for improvements</li> <li>Indicate dates as a</li> </ul>	osed by any complian planned independen accurately as possible	nce schedule or htty of local, Stati e.	any actual da e, or Federal	ites of completion agencies, indica	on for the implem ate planned or ac	entation steps liste	ed below, as applicable	
		Schedule		Actual Complet	tion			
Implementation Sta	age	MM/DD/Y	YYY I				÷	
<ul> <li>Begin construction</li> </ul>	n	1 1		<u>, , ,</u>	<u>r</u>			
- End construction	· ·			12 15	- -	•		•
– Begin discharge	•	05/01/2		<u> / 13 / 2004</u>	<del>4</del> . 			·
- Attain operational	level	05/01/2	005	// //		•		.
e. Have appropriate p	ermits/clearanaa		•		-			
Describe briefly	Environmental A	nceming other F	ederal/State	requirements be	en obtained?	X Yes	No	•
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6. EFFLUENT TESTING D		<u>`</u>						
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BASIC APPLICA	TION INFORMAT			OMB Number 2	040-0086 .
ANT C. CERTIFICA	TION				
All applicants must comple applicants must complete a completed and are submitt hat apply to the facility for	ete the Certification Section. all applicable sections of Form ing. By signing this certification which this application is subn	Refer to instructions to determine w n 2A, as explained in the Application on statement, applicants confirm the nitted	ho is an officer for the pu Overview, Indicate bek at they have reviewed Fo	Irposes of this certification. W which parts of Form 2A m 2A and have completed	All you have
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<u>X</u> Basic Applicat	ion Information packet	Supplemented A - di			
	•	Supplemental Application Informa	ition packet:		• • • • •
		Part D (Expanded Efflue	nt Testing Data)		•
	· · ·	Part E (Toxicity Testing:	Biomonitoring Data)	· · ·	
		Part F (Industrial User Di	scharges and RCRA/CE	RCLA Wastes)	
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Wastewater Treatment Plant Engineering Report

# February 2005

## DRY CREEK RANCHERIA

## WASTEWATER TREATMENT PLANT ENGINEERING REPORT

Prepared For Dry Creek Band of Pomo Indians

Submitted By HydroScience Engineers, Inc. 221 Gateway Road West, Suite 403, Napa, CA 94558

February 2005

#### DTT CREEK BAND OF POMO INDIANS WASTEWATER TREATMENT PLANT ENGINEERING REPORT FEBRUARY 2005 PAGE I OF II

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Appendix A – USGS Gauging Station Data Appendix B – Dry Creek WWTP Improvement Plant Appendix C – Receiving Water Quality Data

## SECTION 1 - PROJECT DESCRIPTION

HydroScience Engineers, Inc. (HSe) was retained by the Dry Creek Band of Pomo Indians (Tribe) to prepare an Engineering Report for the conveyance, treatment, and disposal of wastewater generated by the Dry Creek Wastewater Treatment Plant (WWTP). This plant treats sewage from Tribal facilities on the Dry Creek Rancheria (Project), including the existing River Rock Casino. This document describes the existing Dry Creek WWTP, which currently treats and will continue to treat all of the wastewater generated by the Project. The objectives of this report are to:

- Identify the wastewater treatment options,
- Identify the required wastewater treatment facilities,
- Determine the wastewater treatment plant capacity, and
- Identify the proposed effluent disposal method(s).

### 1.1 Site Description

The Project site is located on the Dry Creek Rancheria in Sonoma County, California. The Project is accessible from Highway 128, and is located on a hillside just east of the Russian River. Figure 1-1 identifies the Project site location. Existing developments within the Rancheria include an entertainment facility (River Rock Casino), parking garages, roadways, utilities, and a tertiary wastewater treatment plant. The entertainment facility contains slot machines, gaming tables and restaurants.

### 1.2 Project Description

This application includes the conveyance, treatment and disposal of wastewater from the Project. The Dry Creek WWTP is located southwest of the existing casino, and produces recycled water for reuse onsite. Recycled water produced on-site is used by the Project for toilet flushing, landscape irrigation, and construction purposes, such as dust control and soil compaction. None of the treated wastewater is discharged to waters of the U.S. (i.e., all of it is recycled on site).

A map of the overall project is included as **Figure 1-2**. A map showing the proposed wastewater treatment and effluent disposal facilities is included in *Section 3*.

### 1.3 Report Organization

This report is divided into three sections as described below.

- Section 1 Project Description: This section provides background project information and a description of the project site.
- Section 2 Proposed Wastewater Treatment: This section identifies the wastewater treatment process, effluent disposal methods, applicable State and Federal laws, historical water quality characterization, current water quality, discharge limit provisions, and anticipated effluent limits.
- Section 3 Wastewater Treatment Plant Process: This section identifies the components, parameters, and the process treatment train for the on-site wastewater treatment plant.





## SECTION 2 – PROPOSED WASTEWATER TREATMENT

This section provides a review of expected Dry Creek WWTP influent water quality, provides an estimate of the quantity of wastewater required for treatment, describes existing wastewater treatment and disposal facilities, and identifies the treatment options explored for this Project.

### 2.1 Influent Water Quality and Capacity

This section provides background on the typical quality of influent water at gaming facilities and identifies the facilities required to treat it.

Water Quality: The quality of the Project's influent wastewater differs somewhat from typical domestic sewage. Typical gaming facility wastes have higher BOD and TSS values compared to domestic wastewater. Typical BOD and TSS values for gaming and domestic sewage are identified in Table 2-1.

### Table 2-1: Typical Dry Creek WWTP Influent Water Quality

Parameter	Units	Dry Creek WWTP	Domestic Sewage
BOD	mg/L	450-600	200-300
TSS	mg/L	450-600	200-300

Shock loadings are also typical of gaming facility wastewater facilities. Weekend flows are much higher than weekday flows, and evening flows are higher than daytime flows. This is largely due to the larger attendance at similar facilities outside of normal business hours: Any wastewater treatment process selected for use must be able to handle the high strength waste and react well to wide variations in flow.

**Capacity:** Average weekday and peak weekend flows were developed from analysis of similar gaming facilities. Based on projected water usage by the Project, daily wastewater demands for weekday and weekend usage are summarized in **Table 2-2**. The average annual flow is a weighted average of the weekday and weekend flows for the Project, and is largely based on historical flows generated from similar gaming facility operations. These numbers are preliminary and are provided for planning purposes only.

### Table 2-2: Design Flows for the Dry Creek WWTP

Average Weekday Flow (gpd)	Average Weekend Flow (gpd)	Average Annual Flow (gpd)
101,000	141.000	112,000
Notes		112,000

Notes:

1. gpd: Gallons per day

2. All flows rounded to the nearest 1,000 gpd.

The existing plant was recently expanded, and has a treatment capacity of approximately 150,000 gpd. Based on the flow projections identified in **Table 2-2**, the Dry Creek WWTP will have sufficient capacity to treat wastewater generated by the Project. DRY CREEK BAND OF POMO INDIANS WASTEWATER TREATMENT PLANT ENGINEERING REPORT FEBRUARY 2005 PAGE 3 OF 25

**Effluent:** This section describes the major water quality constituents of concern in the Dry Creek WWTP effluent. The sources of wastewater contaminants are comprised of those present in the existing groundwater supply and loadings from the Project when the water is used. Some chemical characteristics of wastewater quality vary by location depending on water supplies, while other characteristics such as BOD and suspended solids are based more on the type of use.

Influent wastewater concentrations are summarized in **Table 2-1**. The wastewater is not expected to contain any significant concentrations of heavy metals or other priority pollutants that may be present in municipal treatment plants with industrial dischargers.

Projected effluent quality from the SBR system is summarized in **Table 2-3**. Since construction activities at the Dry Creek WWTP were completed during December 2004, and startup activities are currently underway, representative operating data for the Dry Creek WWTP is not currently available. When operational data is available, it will be submitted to the USEPA in accordance with all permit requirements.

#### Table 2-3: Example SBR Effluent Wastewater Quality

Units	Title 22 <sup>1</sup>	Average
mg/L		3.35
NTU	2 to 5	2.48
MPN/100 ml	2.2	1
	Units mg/L NTU MPN/100 ml	Units         Title 22 <sup>1</sup> mg/L            NTU         2 to 5           MPN/100 ml         2.2

Source: Jackson Rancheria SBR facility.

### 2.2 Existing Dry Creek WWTP Facilities

The existing Dry Creek WWTP was recently upgraded and expanded to a capacity of 150,000 gpd. The Dry Creek WWTP was designed to provide tertiary treatment of wastewater generated by the Project so effluent can be reused on Tribal lands. Effluent water quality is projected to meet or exceed the California Department of Health Services (DHS) standards for unrestricted reuse. It is understood that State regulations do not apply to Tribal lands. A description of each of the following Dry Creek WWTP process components is included below.

- Influent Screening;
- Sequential Batch Reactors (SBRs);
- Filtration;
- UV Disinfection:
- Chlorination/Dechlorination;
- Effluent Pumping:
- Sludge Storage;
- Dewatering:
- Operations Building, and
- Recycled Water Use.

The reader is referred to **Appendix B** for design drawings of each of the existing Dry Creek WWTP process facilities. A wastewater treatment process flow diagram is included as **Figure 2-1**.



#### DRY CREEK BAND OF POMO INDIANS WASTEWATER TREATMENT PLANT ENGINEERING REPORT FEBRUARY 2005 PAGE 4 OF 25

**Influent Screening:** Wastewater from the Project goes through an oil and grease separator, travels via gravity and enters the rotary screen. The rotary screen is self-cleaning and has ¼-inch openings. Particles that do not pass through the screen are transferred to the screening bin, and then trucked off-site. Wastewater then goes to the transfer tank where it is conveyed to the SBR basins for treatment.

**SBRs:** The SBR treatment process is a fill and draw activated sludge treatment system where aeration and sedimentation occur sequentially in batch mode within the same tank. This method of treatment differs from conventional activated sludge systems in that the reactors receive the influent in batches rather than via continuous flow. There are five steps involved in this SBR system:

Step 1.Fill: Screened influent fills the SBR tank until the basin is full. The influent mixes with biomass produced from previous operation of the SBR system.

Step 2.React: Inside the SBR basin, air is added to the mixed liquor via diffusers located in the tank. The air initiates an aerobic reaction where the nitrification process takes place. Mixers ensure distribution of the influent, the food source, to the biomass (organisms) for biological treatment. The biomass is selected to have an MCRT sufficient for partial nitrification and denitrification, though not necessarily at the peak daily capacity. The resulting mixed liquor is then diverted to a second SBR basin, where excess sludge is removed. Once mixing is complete, air is added for polishing as needed.

Step 3. Settle (sedimentation/clarification): All physical activities, e.g. air and mixing, are stopped to allow for settling of the activated sludge.

Step 4. Draw/Decant: Secondary effluent is drawn from the second SBR basin and pumped to a flow equalization tank.

Step 5. Idle: Once the effluent from the SBR basin has been removed, the basin is once again ready for another cycle of treatment.

**Filtration:** Secondary effluent is transferred to a filter flow equalization tank were the effluent is stabilized to provide slower flow rates for the filtering process. Once the effluent is stabilized, it is pumped to one of three Parkson Dynasand sand media filters, each with a surface area of 19 square feet. These filters are continuous upflow filters, in which effluent is pumped upwards through a sand media. Filtered effluent flows over a weir at the top of the filter, and onwards to the UF disinfection unit. The sand inside the filter is continuously backwashed and recirculated back into the media through an air cleaning system. Alum is added to enhance coagulation.

**UV Disinfection:** The filtered effluent flows from the filters to the ultraviolet (UV) units for disinfection. The UV units are housed inside a quartz sleeve and located inside stainless steel channels parallel to the flow. Bacteria are inactivated as the effluent passes by the UV light; preventing them from reproducing. The UV lamps include a self-cleaning system to prevent the build-up of material on the quartz sleeve. The UV system is located opposite the chlorination tank in a newly constructed section of the wastewater treatment facility facing the casino parking lot. There are six UV modules; each one contains ten UV-lamp assemblies.

**Chlorination/Dechlorination:** Filtered effluent that will be utilized for Title 22 approved uses on-site will be pumped from the filters to the chlorine contact tank where chlorine will be added for disinfection. The chlorine contact tank has a 35,200 gallon capacity and is located directly below the filters. When necessary, sodium metabisulphite is used to trim the chlorine residual.

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**Effluent Pumping:** Disinfected effluent is pumped from an effluent pump station through a 6-inch force main to the recycled water storage tanks and distribution system. Each pump has a design set point of 160 gpm at 272 ft total dynamic head.

**Sludge Storage:** Waste sludge from the SBR is pumped to a sludge storage tank where it will be periodically hauled off-site. The sludge storage tank also receives floating and settleable material from the SBR waste process stream and the filter backwash. The tank capacity is 39,200 gallons.

**Dewatering:** The Tribe is currently exploring the option of installing a dewatering system to increase the solids content in the sludge. Preliminary estimates indicate that the centrifuge would produce cakes with a solids content of approximately 16%, therefore, minimizing the number of trucks required to transport the sludge off-site. The centrifuge would be located in a new building adjacent to the sludge storage tank (FWS, July 2004).

**Operations Building:** An operations building contains an office/laboratory, a restroom, a chemical room, and an electrical room. The building is located to the east of the SBR basins behind the filters and chlorination facilities.

**Recycled Water Use:** On-site recycled water use will be maximized, year-round, by using it for landscape irrigation, toilet and urinal flushing, and other approved uses. Additionally, the Tribe will irrigate an existing cemetery and new spray fields with recycled water. All irrigation with recycled water will be within Tribal lands.

To ensure that there is enough capacity to use all recycled water on-site during the summer, when discharge to surface waters is generally prohibited, the Tribe will construct seasonal storage facilities and spray fields to maximize the amount of recycled water that can be used on-site./ Based on a seasonal discharge to surface waters, as described in *Section 3*, approximately 1.8 MG of recycled water storage and 16 acres of spray fields would be constructed at buildout. Currently, the Tribe utilizes multiple Baker tanks for operational recycled water storage, which provide an overall storage capacity of approximately 200,000 gallons. All storage and recycled water use facilities will be located within Trust lands.

All recycled water use would be in accordance with all applicable laws for use of recycled water on Trust lands, as regulated by the Indian Health Services. In addition, the recycled water will meet California Title 22 requirements for unlimited reuse. The use and distribution of recycled water within Trust lands will be further detailed in a separate Engineers Report.

#### 2.3 Plant Design Parameters

The design criteria for the Dry Creek WWTP are summarized in **Table 2-4**. A description of each unit process follows the table.

This section describes the buildout wastewater treatment process proposed for the Project. The reader is referred to **Appendix B** for the Dry Creek WWTP design drawings, which include site layout drawings for each unit process.

## Table 2-4: Dry Creek WWTP Design Parameters

Parameter	Adine
FR Flerre	
Design Flows	450,000 and
Maximum Day Flow:	
Max Hydraulic Day:	300,000 gpa
Reactors	
Volume:	2 @ 92,000 gai (eacn)
Dimensions:	28' x 32'-8" (each)
Sludge Storage Tank	
Volume:	39,200 gal
Dimensions:	12' x 32'-8"
Transfer Tank	1
Volume:	31,000 gal
Filter Flow Equalization T	ank
Volume:	31,000 gal
Filters	
Size:	3 @ 19 sq-ft (each)
Chlorine Contact Tank	
Volume:	35,200 gal
Blowers	
Inlet Air Volume:	670 SCFM, 750 ACFM
Discharge Pressure:	7.5 psi
Motor:	50 HP/1800RPM/460 V/3Ø
Decant Pumps	
Design Point:	385 gpm
Motor:	460 V/3Ø/5 HP
Transfer Pumps	
Design Point:	300 gpm
Motor:	460 V/3Ø/5 HP
Sludge Pumps	
Design Point:	95 gpm
Motor:	460 V/3Ø/2.2 HP
Air Compressor	
Nominal Capacity:	10.9 SCFM @ 90 psi
Motor:	460 V/3Ø/3 HP (each motor)
Туре:	Automatic pressure start/stop 120 gallon horizontal ASME air receiver.
Pressure Cell	
Туре:	Liquid level transmitter
Flow Meter	
Туре:	4" Magmeter
Effluent Pumps	
Design Point:	160 gpm
Motor:	460 V/3Ø/20 HP
Screen	

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DRY CREEK BAND OF POMO INDIANS WASTEWATER TREATMENT PLANT ENGINEERING REPORT FEBRUARY 2005 PAGE 7 OF 25

LV (U.

Parameter Value Motor: 230/480 V/3Ø/3/4"HP Screen Opening: 1/4" **Filter Pumps Design Point:** 140 gpm @ 32-ft total dynamic head Motor: 460 V/3Ø/3 HP Mixer Motor: 460 V/3Ø/4 HP **Recirculation Pump** Design Point: 30 gpm @ 50-ft total dynamic head Motor: 208 V/1Ø/1.0 HP **Odor Control** Motor: 115 V, 1/25 HP

Source: Dry Creek Rancheria Sequencing Batch Reactor (SBR) Water Reclamation Facility Drawings, December 1, 2003.

## 2.4 Surface Water Discharge Options

The Dry Creek WWTP plant produces tertiary effluent which requires seasonal surface water discharge of effluent that cannot be reused on-site. Two receiving waters for surface water discharge have been identified: Stream P1 and Stream A1. Stream P1 would be the primary receiving water, while Stream A1 would be a secondary receiving water. This section will identify the methods for each discharge. The locations of both streams are shown in **Figure 2-2**.

Stream P1: To discharge to Stream P1, the Dry Creek WWTP would convey effluent to an existing stormwater detention basin located to the south and west of the existing plant site. Wastewater from the detention basin would flow through an outlet and down a cascade reaeration system. This system would then drain into an existing ephemeral channel, which is a tributary to Stream P1 within Tribal lands.

Stream P1 flows southeast through the Project site, through several culverts and finally off-site near the southeast corner of the Tribal property. Once off the site, P1 continues on its natural course, crosses Highway 128, then flows into its existing confluence with the Russian River. The confluence of P1 and the Russian River is located at latitude 38°41'27"N and longitude 122°51'31"W, elevation 200-feet, and is located approximately one mile from the Dry Creek WWTP. The location of this discharge and Stream P1 are shown in **Figure 2-2**.

Following on-site reuse of recycled water, surface water discharge to Stream P1 would be the primary method of effluent discharge. Discharge to Stream P1 would be limited both seasonally and based on flow in the Russian River, as further described in *Section 3.5.3*.

**Stream A1**. To discharge to Stream A1, the Dry Creek WWTP would convey effluent in a new pipeline around the entertainment facility to the north of the plant. Effluent would be discharged into an existing intermittent channel within Tribal lands. This proposed discharge location is located at latitude 38°42'19"N and longitude 122°51'36"W. This channel is tributary to Stream A1. The location of this discharge and Stream A1 are shown in **Figure 2-2**.

Stream A1 flows from the discharge location along the northeast border of the trust lands before flowing off Tribal lands to the west. The stream eventually crosses Highway 128, then turns immediately to the

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south within a roadside ditch. This stream terminates in a ditch alongside Highway 128. It is noted that the U.S. Army Corps of Engineers (Corps) has determined that this stream is not tributary to any waters of the U.S., and is considered to be an isolated inland surface water. Thus, when too much water flows in Stream A1, water will sheet flow across an existing vineyard located near the terminus of Stream A1 and on the west side of Highway 128.

Since Stream A1 is not tributary to any stream or waterway (as determined by the Corps), the capacity of this Stream to receive effluent for discharge is limited. All effluent discharge to Stream A1 would either percolate into the ground or evaporate./Effluent discharge from the Dry Creek WWTP to Stream A1 would be limited in volume, year-round, to the flow that would not cause sheet overflow onto the existing vineyard located near the terminus of Stream A1 and on the west side of Highway 128./It was noted that \* studies of Stream A1 by the Tribe have shown that the percolation and evapotranspiration capacity have estimated a maximum capacity of approximately 27,000 during the winter, and 104,000 gpd during the summer (URS, 2004). Background flows in Stream A1 were also calculated in that study to vary from near zero during the summer to up to 1 MGD during the winter.

## SECTION 3 – REGULATORY COMPLIANCE ISSUES

This section discusses the regulatory considerations associated with the treatment, reuse, and disposal of wastewater generated by the Project, including:

#### Proposed disposal methods including:

- On-site land disposal by spray irrigation;
- o On-site reuse of recycled water for landscape irrigation, cooling water, and toilet flushing; and
- Surface water discharge within Trust lands;
- Federal laws, and current water quality compliance issues;
- Provisions of current, local surface water discharge permits; and
- Anticipated effluent limits, receiving water limits, discharge provisions and prohibitions, and monitoring requirements for the proposed Project.

#### 3.1 Dominant Issue

On-site reuse and land disposal by spray irrigation are the preferred dry season disposal alternatives for the Title 22 disinfected tertiary treated effluent. To the extent practical, disposal by these methods will be maximized throughout the year. However, during the wet season, additional disposal capacity is required. Surface water discharge is the preferred supplemental wet season disposal alternative for the Dry Creek WWTP.

The proposed receiving water for the surface water discharge is an unnamed ephemeral stream, referred to as Stream P1 in this document, which is tributary to the Russian River at a point south of the Project site. This proposed discharge location is located at 38°42'06"N and longitude 122°51'31"W (deg, min, sec), and is shown in **Figure 2-2**.

A secondary discharge is an unnamed ephemeral stream, referred to as Stream A1 in this document, which is not tributary to waters of the United States, and terminates in a roadside ditch alongside Highway 128. This proposed discharge location is located at latitude 38°42'19"N and longitude 122°51'35"W, and is also shown in **Figure 2-2**.

In support of the proposed wet season surface water discharge program, the Project will be applying for a NPDES permit, which allows discharges to surface water in accordance with the provisions of the Water Quality Control Plan for the North Coast Region (Basin Plan) and the Federal Clean Water Act. It is understood that the Basin Plan requirements do not apply to Tribal lands. However, the proposed limitations identified in this Section are consistent with the Basin Plan.

#### 3.2 Regulatory Framework

The United States Environmental Protection Agency (USEPA) Region 9 will regulate the surface water discharge, and apply the applicable Federal regulations and standards. The predominant standard USEPA will apply is the Clean Water Act. In addition, the EPA may confer with the North Coast Regional Water Quality Control Board (RWQCB), Region One regarding the state and local regulations, including the Basin Plan.

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This section describes the USEPA authority, the RWQCB Basin Plan, and the requirements of the Clean Water Act.

### 3.2.1 USEPA Authority (Tribal Lands in Trust)

The Federal Clean Water Act (CWA) established a nationwide permit program called the National Pollution Discharge Elimination System (NPDES). The NPDES permit program was established for the purpose of regulating and administering permits for all discharges to receiving waters. In this case, discharge to surface waters is proposed; therefore a NPDES permit will be required.

In some states, the USEPA has delegated the administration of the NPDES permit system to a state agency. In California, USEPA has authorized the State Water Resources Control Board (SWRCB) as the responsible agency for NPDES permits. The SWRCB has further delegated the responsibility for administration of NPDES permits to the various RWQCBs. In delegating the responsibility to the Regional Boards, the State has recognized the differences in water quality requirements within the various regions of the state.

The proposed discharge locations are within Tribal lands. For discharges to surface waters on Tribal lands in California, the USEPA has retained responsibility for issuing NPDES permits. As a result, this discharge is regulated by the USEPA.

## 3.2.2 Water Quality Control Plan for the North Coast Region (Basin Plan)

This section describes the beneficial uses and water quality objectives for the potential receiving waters identified in the RWQCB – Region One Basin Plan

Stream P1: The receiving water, Stream P1, is tributary to the Russian River. Thus, the existing and potential beneficial uses for Stream P1 are considered by the RWQCB to be the same as those for the Russian River. The North Coast RWQCB assigns existing and potential beneficial uses for the Russian River and its tributaries in the Basin Plan, which are listed in **Table 3-1**.

Existing	Beneficial Uses	Potentia	I Beneficial Lisos
MUN	Municipal and Domestic Supply	PPO	
AGR	Agricultural Supply		Industrial Process Supply
IND	Industrial Service Supply	SHELL	Hydropower
GWR	Groundwater Recharge		Acucaulture /
FRSH	Freshwater Replenishment		Aquaculture
NAV	Navigation		
REC1	Water Contact Recreation		
REC2	Non-Water Contact Recreation		
COMM	Commercial or Sport Fishing		
WARM	Warm Freshwater Habitat		
COLD	Cold Freshwater Habitat		
WILD	Wildlife Habitat		

### Table 3-1: Beneficial Uses for the Russian River

HydroScience Engineers, Inc.

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Existing Beneficial Uses		Potential Beneficial Lises	
RARE	Rare, Threatened, or Endangered Species		
MIGR	Migration of Aquatic Organisms		
SPWN	Spawning, Reproduction, and/or Early Development		
Source: Ba	sin Plan 2003 Rev. North Coast Pagian		

Source. Basin Plan, 2003 Rev., North Coast Region.

Existing beneficial uses are uses as they exist at the present time, while potential uses are uses that:

- May have existed prior to November 1975;
- Are attainable via future plans;
- May be classified as an existing use after future review; or
- Are listed as future water quality goals for possible use.

Furthermore, beneficial uses of waters of the State are uses that require protection against water quality degradation by any proposed discharge, and reflect the demands on those water resources. Water quality objectives for the Russian River are based on the identified beneficial uses.

Stream A1: Stream A1 is not tributary to any water body of the United States, as determined by the United States Army Corps of Engineers (ESA, 2004). Thus, the water quality objectives applicable to the Russian River are not applicable to Stream A1.

However, the Basin Plan also identifies specific water quality objectives for other water bodies to prevent the degradation of any existing water body. Specific water quality objectives identified for any new wastewater discharge, in addition to this overall policy, are included in the **Table 3-2**.

## Table 3-2: Water Quality Objectives for Inland Surface Waters

Parameter	Description
Color	Water shall be free of coloration that causes a nuisance or adversely affects beneficial uses.
Taste & Odor	Water shall not contain taste or odor producing substances in concentrations that impart undesirable tastes or odors to fish flesh or other edible products of aquatic origin, or that causes nuisance or adversely affect beneficial uses.
Turbidity	Shall not be increased more than 20% above naturally occurring background levels
Bacteria	In waters designated REC-1, the median fecal coliform concentration on a minimum of not less than five samples for any 30-day period shall not exceed 50 per 100 mL, nor shall more than ten percent of the total samples during any 30-day period exceed 400/100 mL. In waters designated SHELL, the fecal coliform concentration throughout the water column shall not exceed 43 per 100 mL for a 5-tube serial dilution, or 49 per 100 mL for a 3-tube
Temperature	At no time or place shall the temperature of any waters designated COLD or WARM be increased by more than five degrees Fahrenheit.
Chemical Constituents and Radioactivity	For waters designated MUN, chemical constituents and radionuclides shall not be present at levels prohibited by the drinking water standards set forth in Title 22 of the California Code of Regulations.

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PA	GF	12	OF	2

Parameter	Description
Other Parameters	The following are prohibited in concentrations that cause nuisance to or adversely affect beneficial uses: floating material, suspended material, suspended sediment, settleable material, oil and grease, biostimulatory substances.
	Discharges containing toxic substances, pesticides, chemical constituents, or radioactivity in concentrations that impact beneficial uses are prohibited

Source: Basin Plan 2003 Rev. North Coast Region.

Any discharge by the Project to Streams A1 and P1 would be designed to comply with the beneficial uses of that water body and these water quality objectives. It is understood that the Basin Plan requirements do not apply to Tribal lands.

### 3.2.3 Federal Clean Water Act

Under Section 319 of the CWA, States, territories, and authorized Tribes are required, after coordinating input from all stakeholders on a watershed basis, to identify all water bodies that do not meet water quality standards after all point sources of pollution are equipped with the required minimum levels of control technology. The task involves:

- Identification of beneficial uses;
- Assessment of whether or not beneficial uses have been impaired; and
- Identification of the water quality stressor(s) believed to be the cause of the beneficial use impairment, if impairment is recognized.

Section 305(b) of the CWA requires the assembly and submittal to USEPA of a comprehensive analysis of the above elements (a Unified Watershed Assessment) every two years.

Section 303(d) of the CWA requires the States to list surface waters where beneficial uses have been identified in the 305(b) Report as impaired (the 303(d) list). Section 303(d) further requires the establishment of Total Maximum Daily Loads (TMDLs) for all stressors identified in the 305(b) report as contributing to an impairment of a beneficial use.

A TMDL is a numeric target that, when achieved, will result in attainment of water quality standards (non-impairment of beneficial uses). The TMDL includes allocations for all identified sources of the targeted water quality stressor within the watershed.

#### 3.3 Water Quality Characterization of the Russian River

A characterization of the Russian River water quality was prepared utilizing multiple available sources,

- 1. Sample data collected by the Tribe from the receiving surface water Stream P1;
- 2. Laboratory data taken by the Russian River County Sanitation District (Russian River CSD) as required by their NPDES Permit No. CA0024058; and
- 3. Laboratory data collected by the Town of Windsor Wastewater Treatment, Reclamation, and Disposal Facility (Windsor WWTP) as required by their NPDES Permit No. CA0023345.

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In addition, the Russian River at the Geyserville Hydrological Area is currently listed in the 2002 CWA Section for 303(d) List of Water Quality Limited Segment (approved by the USEPA July 2003) for Sedimentation/Siltation and Temperature. The TMDL priority for Sediment/Siltation is medium, while the TMDL priority for Temperature is low.

## 3.3.1 Lab Data – Characterization of Proposed Receiving Waters

The primary unknown regulatory issue associated with the proposed wet season discharge to Stream P1 to the Russian River is the water quality of the Russian River at the confluence with Stream P1.

The Tribe is collecting monthly receiving water quality data downstream of the proposed discharge site on Stream P1. All grab samples are collected approximately 200 yards upstream from the intersection of Highway 128 on Stream P1. Collection of this data will help the USEPA evaluate background water quality, identify potential water quality restrictions, and understand the impacts of the proposed new discharge on the aquatic habitat. The parameters listed in **Table 3-3** were selected for sample analysis to help determine if the proposed surface water discharge would affect existing and proposed beneficial uses of the Russian River.

Parameter	Sample Free	
pH	Sample Frequency	
Temperature	Monthly (lab)	
	Monthly (lab)	
	Monthly (lab)	
155 (mg/L)	Monthly (lab)	
Specific Conductance (umho/cm)	Monthly (lab)	
Hardness (mg CaCO <sub>3</sub> /L)	Monthly (Jab)	
Turbidity (NTU)	Monthly (lab)	
Nitrate (mg-N/L)	Monthly (lab)	
Nitrite (mg-N/L)		
Ammonia (mg-N/L)		
TKN (mg/L)	Monthly (lab)	
Total Phosphorous (ma-P/L)	Monthly (lab)	
Orthophosphato (mg D/L)	Monthly (lab)	
Alkalinity (ma C-00, #)	Monthly (lab)	
Costs and All All All All All All All All All Al	Monthly (lab)	
Carbonate Alkalinity (mg CaCO <sub>3</sub> /L)	Monthly (lab)	
Bicarbonate Alkalinity (mg CaCO <sub>3</sub> /L)	Monthly (Jab)	<u> </u>
Hydroxide Alkalinity (mg CaCO <sub>3</sub> /L)	Monthly (lab)	
Total Coliform (MPN/100 mL)	Monthly (lab)	
Fecal Coliform (MPN/100mL)	Monthly (IdD)	
Oil and Grease (mg/L)		
	monthly (lab)	

Table 3-3: Proposed Receiving Water Quality Baseline Monitoring Program

Two separate samples will be collected from each proposed receiving water and analyzed for Trace Metals and California Toxics Rule pollutants. There are 126 pollutants and approximately 40 Trace Metals that these two samples will be tested for, as identified in **Table 3-4**.

Laboratory Test	Laboratory Analysis Method
Volatile Organics	FPA 624
Semivolatile Organics	EPA 625
Pesticides & PCBs	EPA 608
Polynuclear Aromatic Hydrocarbons	EPA 610
Organophosphorus Pesticides	EPA 614
Low Level Mercury	EPA 1631
Metals by EPA 6020/200.8	EPA 6020/200 8
Cyanide, total	EPA 335.2
TriButyl Tin	GCEPD
EPA 1613 2,3,7,8 TCDD (Dioxin)	EPA 1613
Asbestos TEM	
Chromium, hexavalent (colorimetric)	
	LFA/190

## Table 3-4: California Toxics Rule and Trace Metals Laboratory Tests

## 3.3.2 Analytical Results – Preliminary Water Quality Summary

To date, one receiving water quality sample has been collected from both Streams A1 and P1 and tested for all of the parameters and constituents identified in **Tables 3-3** and **3-4**. A complete list of this data is included as **Appendix C**. The Tribe is in the process of collecting additional samples to obtain at least six months of receiving water quality for all of the conventional parameters listed in **Table 3-3**, and two separate samples for all of the CTR and Trace Metal constituents identified in **Table 3-4**. The data below summarizes key issues identified by the existing data.

The completed sample of Stream A1 identified eight constituents at concentrations above the most stringent pertinent water quality criterion: Aluminum, Cadmium, Chromium, Copper, Iron, Lead, Nickel, and Zinc. These criteria included the National Toxics Rule, California Toxics Rule, USEPA criteria, the Basin Plan, and the California Department of Health Services Primary MCL. It should be noted that this sample was collected during the first flush of the stream. During the first flush, receiving water concentrations can be higher than normal, since the receiving water is receiving its first significant rainfall. This can result in increased amounts of non-point source pollution occurring in the receiving water.

In addition, three constituents in Stream P1 were present at concentrations above the most stringent pertinent water quality criterion: Aluminum, Hexavalent Chromium, and Copper. Though no specific criterion is identified for Hexavalent Chromium, this constituent is a known carcinogen, and is presented here for illustrative purposes.

A description of each of the constituents exceeding a water quality criteria is summarized in Table 3-5.

Parameter	Concentration (ug/L)	MDL (ug/L)	Reporting Limit	Water Quality
Stream A1		1	(	Onterion (ug/L)
Aluminum	21,000	30	50	1.000 (DUIO)
Cadmium	1.0	0.11	0.50	0.77 (DHS)
Chromium	71	0.51	10	0.77 (EPA) 47 (CTR) 50 (Basin Plan)
	110	1.6	10	6.4 (CTR) 8.1 (EPA)
iion	30000	51	100	NA
Lead	86	0.69	3.0	1.6 (EPA, CTR) 50 (Basin Plan)
Nickel	53	1.3	10	37 (CTP)
	160	2.8	20	74 (EPA)
Stream P1				03 (CTR)
Aluminum	4,300	30	50	1.000 (DU0)
Hexavalent Chromium	3.5	NA	0.50	1,000 (DHS)
Copper	13	1.6	10	 6.4 (CTR) 8.1 (EPA)

# Table 3-5: Summary of Water Quality Criterion Exceedances – Priority Pollutants and Other Constituents

Additionally, during December 2003 and January 2004, a sample was collected from both Streams A1 and P1. The location this sample was collected from is unknown, and may not be representative of the actual receiving water quality. Both data sets are included in **Appendix C**.

## 3.3.3 Lab Data – Russian River CSD and Windsor WWTP

In lieu of having a comprehensive summary of receiving water quality data for each of the two proposed receiving waters, available water quality data near the proposed Stream P1 and Russian River confluence was collected and summarized. This data was obtained from the monitoring results compiled by two nearby plants, the Russian River CSD and the Windsor WWTP. These plants are located approximately 16 and 18 miles downstream of the Stream P1 and Russian River confluence, respectively.

Available data for the Russian River CSD and for the Windsor WWTP are included in **Table 3-6** and **Table 3-7** respectively. This data spans between January 1996 and May 2004 for the Russian River CSD, and November 2001 through April 2004 for the Windsor WWTP. Average values of this data are presented in the tables. A complete list of this data can be found in **Appendix C**.

#### Table 3-6: Russian River CSD Receiving Water Quality Data

Parameter	Units	Upstream	Downstream
Hardness	mg CaCO3/L	109.7	108.1
Turbidity	NTU	36.3	35.4
Dissolved Oxygen	mg/L	10.3	9.9
pH		7.9	7.9
Notes:			

The upstream sample location refers to Vacation Beach, which is approximately 1,000 feet upstream of the RRCSD 1., discharge to the Russian River.

The downstream sample location refers to the Northwood Golf Club, which is approximately 300 feet downstream from the 2. RRCSD discharge to the Russian River.

Samples represent an average of receiving water data collected between January 1996 and May 2004. 3.

#### Table 3-7: Windsor WWTP Receiving Water Quality Data

Parameter	Units	Upstream	Downstream
Ammonia Nitrogen	mg/L	<0.2	<0.2
Unionized Ammonia	mg/L	<0.1	<0.1
Nitrate Nitrogen	mg/L	1.0	1.0
Organic Nitrogen	mg/L	1.5	1.0
Total Phosphate	mg/L	1.6	1.6
BOD <sub>5</sub>	mg/L	3.1	2.9
Nonfilterable Residue	mg/L	28.4	28.0
Temperature	°C	14.1	13.8
Dissolved Oxygen	mg/L	9.4	97
pН		72	72

The upstream sample location refers to the sample location upstream of the existing discharge at the control valve site at the 1. intersection of Trenton-Healdsburg Road and Mark West Section Road

The downstream sample location refers to the sample location downstream of the existing discharge at the control valve site 2. at the intersection of Trenton-Healdsburg Road and Mark West Section Road

Samples represent an average of receiving water data collected between November 2001 and April 2004. 3.

#### 3.3.4 Discussion

The proposed Dry Creek WWTP discharge is not expected to cause or contribute to any excursions from any existing water quality criteria or standards. The Tribe is and will continue to collect and present to USEPA additional receiving water quality data to supplement this information. Available water quality data for the Russian River near Stream P1 has been collected and summarized in this document.

The receiving waters at the two downstream plants, the Russian River CSD and the Windsor WWTP, do not appear to be in excess of any water quality criterion or standard for which numeric standards have been set. Initial reports showed excursions to water quality criteria for certain metals in Streams A1 and P1, as previously described. However, additional samples will be collected in order to confirm this impression.

#### Existing Dry Creek WWTP Effluent Water Quality 3.4

As mentioned previously, since construction activities at the Dry Creek WWTP were completed during December 2004 and startup activities are currently underway, actual operating data representative of the Dry Creek WWTP is not currently available. When operational data is available, it will be submitted to the USEPA in accordance with all permit requirements.

The reader is referred to Table 2-3 for an example of effluent water quality for a similar SBR treatment plant.

#### Existing Local State Discharge Permits 3.5

The current NPDES permits for the previously mentioned Windsor WWTP and the Russian River CSD were reviewed to gain a sense of the types of operating requirements currently being imposed by the North Coast RWQCB. These plants are the two nearest and most applicable wastewater treatment plants to the Dry Creek WWTP. Both permits include surface water discharge to the Russian River or its tributaries, tertiary treatment, and seasonal discharge.

The types of permit conditions discussed below include:

- Effluent Limitations,
- Receiving Water Quality Limitations, and
- Waste Discharge Rate Limitations.

#### 3.5.1 Effluent Limitations

For water intended for either reclamation or discharge to a surface water, the following effluent limitations were implemented by the RWQCB for the Windsor WWTP and Russian River CSD.

Table 3-8: Local NPDES Effluent Limitations for Reclamation and Discharge to Surface	
Surface to	e Waters

Constituent	Units	Daily Maximum	Weekly Average	Monthly
BOD (20°C, 5-day) (Windsor WWTP)	mg/L	20	15	10
BOD (20°C, 5-day) (RRCSD – discharge)	ma/L		15	10.
BOD (20°C, 5-day) (RRCSD – reclamation)	ma/l		15	-10
Total Suspended Solids (Windsor WWTP)	mg/L	20	43	- 30
Total Suspended Solids (RRCSD - reclamation)	mg/L	20	15	10
Total Suspended Solids (RRCSD - discharge)			45	30
Total Coliform <sup>a</sup>	MDN/400		15	10
Chloroform (RPCSD)	MPN/100 ml	240	23	2.2
	µg/L			100
Dichlorobromomethane <sup>4</sup> (RRCSD)	hð\r	1.12		0.56

Total Coliform Daily maximum refers to maximum for any sample. Total Coliform weekly mean refers to concentrations shall not exceed that value more than once in any 30-day period. Total coliform monthly mean concentrations refer to the seven-day median concentration.

- 2. Effluent limitations for mass loadings are excluded from this table, due to the differences in flows between those plants and the Dry Creek WWTP.
- 3. Sources are Orders No. R1-2003-0026 (Russian River CSD) and R1-2002-0013 (Windsor WWTP).
- 4. The Russian River CSD had an interim limitation of 32 ug/L for this parameter.

Chloroform and Dichlorobromomethane are included in the Russian River CSD permit and not the Windsor WWTP permit based on site specific monitoring for priority pollutants promulgated by the USEPA through the National Toxics Rule and California Toxics Rule. Russian River CSD effluent and ambient monitoring data was analyzed with a reasonable potential analysis, and the RWQCB determined that water quality based effluent limits for both constituents were appropriate. No parameters were included in the Windsor WWTP NPDES permit based on similar methodology.

**Turbidity:** Turbidity limitations also exist in the NPDES permits for the Windsor WWTP and the Russian River CSD (for reclamation use only). These permits mandate that the turbidity of the filtered wastewater shall not exceed the following limitations:

- An average of 2 NTU within a 24-hour period;
- 5 NTU more than 5 percent of the time within a 24-hour period; and
- 10 NTU at any time.

In addition, the effluent discharge shall not cause the turbidity of the receiving water to increase by more than twenty percent above naturally occurring background levels.

**Temperature:** The Russian River CSD NPDES permit simply states that the discharge will not alter the natural temperature of the receiving water. The Windsor WWTP discharge permit places specific limitations on the temperature range allowed in the receiving water, as summarized below.

- When the receiving water (temperature) is below 58°F, the discharge shall cause an increase of no more than 4°F in the receiving water, and shall not increase the temperature of the receiving water
  beyond 59°F. No instantaneous increase in the receiving water temperature shall exceed 4°F at any time;
- When the receiving water (temperature) is between 59°F and 67°F, the discharge shall cause an increase of no more than 1°F in the receiving water. No instantaneous increase in receiving water temperature shall exceed 1°F at any time; and
- When the receiving water (temperature) is above 68°F, the discharge shall not cause an increase in the temperature of the receiving water.

Other limitations relating to effluent water quality and not summarized in this section are described in the following section.

### 3.5.2 Receiving Water Limitations

The following limitations are included in current local state permits, typically to prevent nuisances and protect beneficial uses.

• If the ambient DO concentration in the receiving waters is less than 7.0 mg/L, then the receiving water limitation in current permits is that the discharge shall not depress the DO in the receiving waters below the existing ambient value.

- The discharge shall not cause the pH of the receiving waters to be depressed below 6.5 nor raised above 8.5. Within this range, the discharge shall not cause the pH of the receiving waters to change more than 0.5 units at any time from the naturally occurring pH. If the pH of the receiving water is less than 6.5, the discharge shall not cause a further depression in the pH of the receiving water. If the pH of the receiving water is greater than 8.5 the discharge shall not cause a further increase in the pH of the receiving water.
- Settleable solids shall not be present in a measurable amount in the effluent.
- The discharge shall not cause the receiving waters to contain oils, greases, waxes, or other materials in concentrations that result in a visible film or coating on the surface of the water or on objects in the water.
- The discharge shall not cause the receiving waters to contain floating materials, including solids, liquids, foams, and scum.
- The discharge shall not cause coloration of the receiving waters.
- The discharge shall not cause the receiving waters to contain taste or odor producing substances in concentrations that impart undesirable tastes or odors to fish flesh or other edible products of aquatic origin.

Additional standard receiving water limitations are included in all NPDES permits for discharges to the Russian River or its tributaries. These limitations include numeric and narrative water quality objectives which are set forth in the Basin Plan, and are discussed in *Section 3.2.2*. A list of the potential receiving water limitations for the proposed Project is presented in *Section 3.6.1*.

#### 3.5.3 Waste Discharge Rate Limitations

For the Russian River and it tributaries, the Basin Plan limits direct discharges of treated municipal wastewater to a flow rate that is a maximum of 1% of the receiving water flow rate, unless RWQCB grants an exception to the waste discharge rate limitation. Where practical, the receiving water flow rate of the receiving water is measured as close as possible to the permitted discharge point. However, in some cases, flow rate measurement infrastructure is not located at the permitted discharge point, and the receiving water flow rate measurements for implementation of the waste discharge flow rate limitation are made thousands of feet, or even miles, away from the permitted discharge point. Below are examples of wastewater treatment facilities which do not measure receiving water flow rate at the same location as their permitted discharge location.

**Russian River CSD:** The permitted discharge point is on the Russian River. The receiving water flow rate measurement location used to determine their allowable discharge is the USGS gauge No. 11-4670.00 at the Hacienda Bridge. This gauging station is approximately eight miles upstream from the point of discharge, but is the most representative of the flow rate in the Russian River at the point of discharge.

**Windsor WWTP:** The permitted discharge point is between the effluent storage pond system and Mark West Creek. The receiving water flow rate measurement location is governed by flow conditions in Mark West Creek measured at the Trenton-Healdsburg Bridge, and is limited to 1% of the natural flow in the creek minus the discharge flow of wastewater from the City of Santa Rosa's Laguna Subregional Wastewater Treatment Facility. This gauging station is approximately six miles downstream from the point of discharge.

**Forestville CSD:** The permitted discharge point is on Jones Creek approximately one half mile from the confluence of Jones Creek and Green Valley Creek. The receiving water flow rate measurement location used to determine the Forestville CSD allowable effluent discharge flow rate is located on Green Valley Creek at the Iron Horse Bridge, which is approximately one half mile upstream from the Jones Creek confluence, and approximately one linear stream mile away from the permitted discharge point.

Laguna Subregional Wastewater Facility: The main discharge locations are on the Laguna de Santa Rosa and on Santa Rosa Creek, at locations approximately 0.5 mile and 7 miles from the wastewater plant, respectively. The receiving water flow rate measurement location used to determine the allowable effluent discharge flow rate is located on the Russian River at the Hacienda Bridge, which is approximately eight miles downstream from the Santa Rosa Creek discharge point, and approximately 14 miles downstream from the Laguna de Santa Rosa discharge point.

## 3.6 Anticipated Project Wastewater Discharge Permit

The conditions expected in a NPDES discharge permit for the proposed Project were developed by considering federal regulations, the Basin Plan, other current local discharge permits, and the current regulatory climate. The greatest weight was placed on the most recently adopted local permits, and on permits involving discharges to the Russian River.

This section identifies anticipated effluent limitations, receiving water limitations, provisions and prohibitions, monitoring requirements, and water reclamation requirements.

## 3.6.1 Anticipated Effluent Water Quality Limitations

Based on the most recently adopted local permits and the requirements of federal regulations, the following effluent limitations are expected for the proposed wastewater disposal options.

Table 3-9: Anticipated Effluent Limitations for Reclaimed Water Intended for Unrestricted Use, Requiring Tertiary Treatment and Disinfection

Constituent	Units	Daily Maximum	Weekly Mean	Mandhlu Marrie
BOD (20 deg C, 5-day)	Units /) mg/L ds mg/L MPN/100 ml mg/L		Weekly mean	wonthly wean
Total Susponded Selida			45	30
Total Suspended Solids	mg/L		45	30
		2.2	7-day med	ian
otal Coliform	MPN/100 ml	23	No more the any 30-day	nan one sample in
		240	No sample	
Settleable Solids	mg/L		None	

### Table 3-10: Anticipated Effluent Limitations for Surface Water Discharge

Constituent	Units	Daily Maximum	Weekly Mean	Monthly Mean
BOD (20 deg C, 5-day)	mg/L	20	15	10
Total Suspended Solids	mg/L	20	. 15	10
Turbidity	NTU	5 <sup>1</sup>	-	2

Note:

1. It is expected that excursions will be allowed if less than 5% of the time in any 24-hour period, but not allowed at any time to exceed 10 NTU.

In addition, the Title 22 requirements for UV disinfection effectiveness and reliability are expected to be incorporated.

### 3.6.2 Anticipated Receiving Water Quality Limitations

Numerical receiving water quality limitations are likely to include the limitations listed in Table 3-11.

Parameter	Receiving Water Limits
Dissolved Oxygen	The discharge shall not cause the DO concentration in the receiving waters to be depressed below 7.0 mg/L. If the ambient DO concentration in the receiving waters is less than 7.0 mg/L, then the discharge shall not depress the DO in the receiving waters below the existing ambient value.
pH	6.5 minimum, 8.5 maximum
	<ul> <li>When the receiving water is below 58°F, the discharge shall cause an increase of no more than 4°F in the receiving water, and shall not increase the temperature of the receiving water beyond 59°F. No instantaneous increase in receiving water temperature shall exceed 4°F at any time.</li> </ul>
Temperature	<ul> <li>When the receiving water is between 59°F and 67°F, the discharge shall cause an increase of no more than 1°F in the receiving water. No instantaneous increase in receiving water temperature shall exceed 1°F at any time.</li> <li>When the receiving water is above 68°F, the discharge shall not cause an increase in</li> </ul>
	temperature of the receiving water.

### Table 3-11: Anticipated Receiving Water Limitations

In addition, the proposed discharge is likely to be subject to the following narrative receiving water limitations:

- The discharge shall not cause the turbidity of the receiving waters to increase by more than 20 percent above naturally occurring background levels.
- The discharge shall not cause the receiving waters to contain floating materials, including solids, liquids, foams, and scum, in concentrations that cause nuisance or adversely affect beneficial uses.
- The discharge shall not cause the receiving waters to contain taste or odor-producing substances in concentrations that impart undesirable tastes or odors to fish flesh or other edible products of aquatic origin, that cause nuisance, or that adversely affect beneficial uses.
- The discharge shall not cause aesthetically undesirable discoloration of the receiving waters.
- The discharge shall not cause bottom deposits in the receiving waters to the extent that such deposits cause nuisance or adversely affect beneficial uses.

- The discharge shall not contain concentrations of bio-stimulants that promote objectionable aquatic or algal growths to the extent that such growths cause nuisance or adversely affect beneficial uses of the receiving waters.
- The discharge shall not cause the receiving waters to contain toxic substances in concentrations that are toxic to, degrade, or that produce detrimental physiological responses in humans or animals or cause acute or chronic toxicity in plants or aquatic life.
- The discharge must not cause bioaccumulation of pesticide, fungicide, wood treatment chemical, or other toxic pollutant concentrations in bottom sediments or aquatic life to levels that are harmful to human health.
- The discharge must not cause the receiving waters to contain oils, greases, waxes, or other materials in concentrations that result in a visible film or coating on the surface of the water or on objects in the water that cause nuisance or that otherwise adversely affect beneficial uses.

## 3.6.3 Anticipated Provisions and Prohibitions

The anticipated provisions and prohibitions for the Project's NPDES permit are presented in this Section.

Seasonal prohibition: For the Russian River and it tributaries, point source direct discharges of treated municipal wastewater are restricted to the period between October 1 and May 14. No known exceptions to this rule have been recently promulgated. It is expected that the Project's NPDES permit will include this restriction with regards to P1 discharge. We

Maximization of reuse: The on-site water reclamation facilities will be operated to implement all reasonable alternatives for reclamation, and to limit the portion of facility effluent that is discharged to surface waters to the lowest percentage practicable.

Waste discharge flow restriction: For the Russian River and it tributaries, direct discharges of treated municipal wastewater are restricted to a flow rate that is a maximum of 1% of the Russian River flow rate. ADEL COM

The amount of effluent discharge allowed by the Basin Plan is typically limited to a percentage of the measured streamflow in the Russian River at the point of discharge. In all local discharge permits reviewed in this document, the existing USGS flow gauging station most representative of the flow in the receiving water was used for the purposes of complying with Basin Plan mandated limitations for flow. There are no existing gauging stations on either Stream A1 or P1. The nearest three USGS gauging stations are located at the following locations:

- USGS gauging station #11463200 16 miles upstream near Cloverdale, 1.
- 2. USGS gauging station #11463980 15 miles downstream near Healdsburg at Digger Bend, and
- USGS gauging station #11464000 18 miles downstream near Healdsburg.

Gauging station #1463980, at Digger Bend, is the station closest to the Project site. However, historical data from this gauging station for February and March (wet season) are not available. Data for the next two closest gauging stations, gauging station #11463200, and gauging station #11464000, are available to estimate the flows near the confluence of Stream P1 and the Russian River.

A conservative approach to implementation of compliance with the Basin Plan discharge flow limitation would be to limit the discharge to P1 from the Dry Creek WWTP to 1% of the measured flow in the

Russian River at Cloverdale, the upstream gauging station. The location for this gauging station is shown in **Figure 2-3**.

If the USEPA grants the Tribe permission to monitor flow at the USGS gauging station at Cloverdale, it is anticipated that the Tribe will be able to comply with the 1% flow restriction without any allowances, although on-site effluent storage outside of the permitted discharge period will likely be required. If permission is not granted, it is not known how flow would be monitored, since there are no existing stream gauges with historical flow data at on this stream. Additional information would need to be obtained and presented.

## 3.6.4 Anticipated Monitoring Requirements

Based primarily on the Monitoring and Reporting Program (MRP) for the Windsor WWTP, routine monitoring frequency and type of sample collected for the Dry Creek WWTP influent will likely be as presented in **Table 3-12**.

## Table 3-12: Anticipated Dry Creek WWTP Influent Monitoring Requirements

Parameter	Units	Type of Sample	0
Flow (mean and poak)		Type of Sample	Sampling Frequency
now (mean and peak)	mgd	Meter	Continuous
BOD (20°C, 5-day)	mg/L	8-hr Composite	
Non-filterable Residue			vveekiy
	mg/L	8-hr Composite	Weekly

Based on the MRP for the Laguna Subregional Wastewater Facility, routine monitoring frequency and type of sample collected for the effluent will likely be as presented below in **Table 3-13**. All of these parameters would be sampled for only during the period which effluent is discharged to a surface water.

## Table 3-13: Anticipated Dry Creek WWTP Effluent Monitoring Requirements

Parameter	Units	Type of Sample	0			
BOD (20°C 5-day)		i jpe of Sample	Sampling Frequency			
Total Suggestind D. K.	mg/L	8-hr Composite	Weekly			
Total Suspended Solids	mg/L	8-hr Composite	Weekly			
Settleable Solids	ml/L	Grah	Weekiy			
pH	nH   Inite	Grab	VVeekly			
Total Coliform	prionits	Grab	Daily			
	MPN/100mL	Grab	Daily			
UV Transmittance	Percent	Meter	Continuous			
Operational UV Dose	mW-s/cm	Coloulatio	Continuous			
Chlorine Residual		Calculation	30-min Intervals			
Flow (Maga & D. L.)	mg/L	Meter	Continuous			
riow (iviean & Peak)	mgd.	Meter	Continuouo			
Turbidity	NTU	Motor	Continuous			
Priority Pollutants		ivieter	Continuous			
y ondanto	Various	24-hr Composite				

Based on the MRP for the Windsor WWTP, routine monitoring frequency and type of sample collected for the receiving water, upstream and downstream from the point of discharge, will likely be as presented in **Table 3-14**.

Parameter	Units	Type of Sample	
BOD (20°C 5-day)		- Type of Sample	Sampling Frequency
<u>See (20 0, 0 day)</u>	mg/L	Grab	Weekly
Suspended Solids	mg/L	Grab	M/ookly.
Settleable Solids	ml/l	Cart	VVEEKIY
Dissolved Oxygen		Grao	Weekly
	mg/L	Grab	Daily
Hydrogen Ion	pH	Grab	 D=!!
Priority Pollutants	Various		Dally
	various	Grab	Annually

## Table 3-14: Anticipated Receiving Water Monitoring Requirements

All analytical methods would conform to the most applicable laboratory method identified either in Standard Methods or by the USEPA.

## 3.6.5 Anticipated Water Reclamation Requirements

It is expected that the NPDES permit will stipulate similar requirements as described in the water reclamation provisions of the California Code of Regulations, Title 22, Division 4, Chapter 3. It is understood that State regulations do not apply to Tribal lands.

It is expected that the disposal by land irrigation will be limited to agronomic rates as estimated by local evapotranspiration data, and in consultation with the rules and regulations required by the California Department of Health Services and regulated by the Indian Health Services.

#### DRY CREEK BAND OF POMO INDIANS WASTEWATER TREATMENT PLANT ENGINEERING REPORT FEBRUARY 2005 PAGE 25 OF 25

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- 13. United States Geological Survey, Daily Streamflow for the Nation USGS 11463200 BIG SULPHUR
- C NR CLOVERDALE CA, http://nwis.waterdata.usgs.gov/nwis/discharge/?site\_no=11463200&agency\_cd=USGS, 1957 - 2003.
- 14. URS, Alternative Discharge Route Evaluation email, April 2004.

## **APPENDIX A**

USGS – Monthly Streamflow Data for Gauging Stations #11463200, #11463980, and #11464000.

HydroScience Engineers, Inc.

## Monthly Streamflow Statistics for the Nation USGS 11463200 BIG SULPHUR C NR CLOVERDALE CA

Sonoma County, California Hydrologic Unit Code 18010110 Latitude 38°49'34", Longitude 122°59'45" NAD27 Drainage area 85.5 square miles

YEAR		Monthly mean streamflow, in ft <sup>3</sup> /s												
	Jan	Feb	Mar	Apr	May	/ Jun	Jul	Auc	Sen	Oct	Nov	Dee		
										1 000		Dec		
1957	N	D N	D N	D N	D N	D N	D 18	.6 8.2	2 51	4 18	6 10	8 202		
1958	51	8 1,96	2 74	7 72	6 79	.4 45	.8 19	9 8.8	4 5 5	8 53	5 75	292		
1959	44	7 69	7 98.	5 45.	8 21	.9 1	1 4.2	3 3.1	3 11	2 49		5 6 9 2		
1960	14	5 65	8 39	8 10	5 58	6 23	9 8.6	4 3.9	9 37	8 59	5 54	1 202		
1961	20	5 39	7 29	9 12	6 58	7 25.	2 7.6	7 5.0	4 4 1	6 44	3 12	3 190		
1962	9	4 94	0 39	2 83.	1 35.	7 16.	9 6.7	2 5.1	3 4.1	6 29		6 330		
1963	39	1 58	6 33	0 68	6 17	5 5	4 2	2 1	1 87	8 17	5 27			
1964	32	3 81.	7 66.	4 37.	9 22.	2.9 13.2 5.9		4 3.2	2 29	6 56	0 15	8 1 2 2 9		
1965	99	5 15	5 75.	2 43	7 84.	5 28.	3 14.	3 8.1	8 59	6 61	1 16	0 1,220		
1966	794	4 410	0 17	4 87.:	3 32.	5 15.	6 8.2	2 4.6	6 37		8 28	2/4		
1967	769	9 268	3 43	3 449	9 12	1 6	7 16.	4 11	9 8	$\frac{-4.0}{2}$	1 1	090 B 140		
1968	544	4 47	7 26	9 87.2	2 39.	1 16.	9 7.0	9 7	1 5.5	2 10	7 22	1 366		
1969	1,395	5 1,176	34	3 187	62.	5 29.	2 11.	7 7.1	7 5.1	5 8 46	11 0	834		
1970	1,971	444	4 276	61.6	§ 29.	5 14.	2 5.8	3.5	5 2.79	7 50	26	1 006		
1971	472	86.2	2 288	3 125	6 47.	1 2	2 8.8	5 5.92	4 36		12			
1972	105	5 159	102	2 116	§ 30.9	9 11.9	5.23	3 3.14	3 54					
1989	ND		NE NE	ND ND	N		) NC		5.35	N	24 0	12 6		
1990		ND	66.5	5 31.5	NE	NE NE	15.2	2 5.4	3.31	3.31	6.21	5 95		
1991	5.66		ND	ND ND	37.3	3 16.3	6.95	5 3.37	2.42	3.80	0.2			
1992		ND ND	ND	ND	24.1	13.7	1 11	1.79	2.16	10.6	0.0			
1993		ND	ND	ND	ND	ID 79 21.6 9.64 5.8 6		9.64 5.8		12 3				
1994	ND	<u>ND</u>	76.5	40.6	25.3	25.3 9.76 3.17 1.52 1.83		1.52 1.83		33 5				
1995	<u>ND</u>	<u>ND</u>	ND	ND	ND	48	28.2	11.4	11.4 6.58		6 35	I ND		
1996		ND	ND	ND	ND	42.3	18.6	6.94	6.94 5.2		ND			
1997	ND ND	ND	ND	62.3	17.6	10.6	5.04	2.06	2.53	4.97	ND			
1998		ND	ND	ND	ND	ND	39	15.6	9.01	8.51	25.2	ND		
1999		ND	ND	ND	50.5	22.9	11.6	6.9	3.8	5.38	ND	36.3		
2000			ND	ND	56.5	23.2	10.2	4.45	3.91	11.2	13	17.4		
2001		ND	ND	46	18.8	6.65	2.85	1.22	1.62	3.41	ND	ND		
2002	ND ND 4		40	30.2	10.5	3.82	2.24	1.78	2.22	26.1	ND			
2003		ND		ND	ND	33.2	14.8	7.62	4.85	. ND	ND	ND		
Average (cfs)	age (cfs) 573.4 566.5 261.1 17		179.0	50.4	26.3	12.1	6.0	6.2	23.1	66.6	300 E			
Average (MGD)	D) 370.5 366.1 168.8 115.		115.7	32.6	17.0	7.8	3.9	4.0	15.0	42.0	200.1			
5% Of Average		[								13.0	43.0	200.1		
(MGD)	18.53	18.30	8.44	5.78	1.63	0.85	0.39	0.19	0.20	0.75	-2 1E	10.00		
(MGD)	3.71	3.66	1.69	1 16	0.22	0.47	0.00		0.20	0.75	2.13	10.00		
		5.66	1.00	1.10	0.33	0.17	0.08	0.04	0.04	0.15	0.43	2.00		

#### ND: No Data

Note: ND readings are not included in calculating average flows.

# Monthly Streamflow Statistics for California USGS 11463980 RUSSIAN R A DIGGER BEND NR HEALDSBURG CA

Sonoma County, California Hydrologic Unit Code 18010110 Latitude 38°37'59", Longitude 122°51'16" NAD27 Drainage area 791 square miles

YEAR	:	·		• •									
	Jan	Feb	Mar	Apr	Ma	v Jun		mow, I	nπ'/s				
						<u>, ou</u>	J		lg s	Sep	Oct	No	/ Dec
1987	N	D.N	D N	D N						_			
1988	N	D NI	D N	D N						ND	17	0 18	37 ND
1989	N	D NI	D N				25 9	.3	05	103	11	2 N	
1990	N			10			14 1	96 2	213	202	N	D N	D 209
1991	97.	3 N					<u>D 1</u>	65 1	73	146	N	D N	D 116
1992	NE					<u>U 14</u>	5 1	49 1	62	164	16	4 13	5 ND
1993	NE					<u>U 21</u>	2 2	08 2	02	180	190	3 18	
1994	1 NE							D 2	08	173	NE	) NI	
1995	ND				18	6 11	2 10	)9 1	26	162	102		
1996	ND	ND					D 23	88 2	22	205	195	10	
1997			INL NE				21	6 2	04	183	228	NIT	
1998		ND				2 18	9 17	5 1	78	189	186		
1999	ND				NE		) N	2	26 3	280	252		
2000		ND	ND	ND	NE	264	1 21	6 23	12 2	220	102	INL.	
2001			<u>ND</u>	ND	ND	235	5 19	7 10	15 1	85	100	NL	ND
2007		ND	ND	ND	188	117	97	2 96	7 1	52	190	194	237
2002		ND	ND	ND	301	203	18	15	5 1	55	137	ND	ND
2005	ND ND ND ND		ND	ND	242	2 21		20	158	<u>ND</u>	ND		
Average (cts)	97.3	ND	ND	193	225	181.6	177 5	192 0	400			ND	ND
5% of Average	62.88 ND ND 124		124.7	145.4	117.4	1147	102.2	180	4	175.5	179.6	187.3	
3 /0 OF Average			1				114.7	117.8	116	.6 1	13.4	116.1	121.1
1% of Average	3.14	ND	ND	6.24	7.27	5.87	574	5.90				· · ]	
(MGD)	0.62						0.14	5.69	5.8	<u>s   (</u>	<u>5.67</u>	5.80	6.05
(	0.63	ND	ND.	1.25	1.45	1.17	1.15	1 18	1.15	,   .			
									1 1.17	- i -	1131	1 16	1 0 1

ND: No Data

Note: ND readings are not included in calculating average flows.

## Monthly Streamflow Statistics for California USGS 11464000 RUSSIAN R NR HEALDSBURG CA

Sonoma County, California Hydrologic Unit Code 18010110 Latitude 38°36'48", Longitude 122°50'07" NAD27 Drainage area 793 square miles Gage datum 77.01 feet above sea level NGVD29

YEAR				N	onthly	mean st	reamflo	w. in ft	<sup>3</sup> /c				
	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	
4000							· ·				1		
1939					ND	ND	ND	ND	ND	180	14	387	
1940	4,52/	9,205	4,276	1,925	452	199	149	108	174 187		27	5 342	
1941	6,962	6,631	4,684	4,823	787	372	195	185	205	208	306	5 584	
1942	4,672	1,7,746	1,392	2,874	957	414	. 194	132	. 128	146	478	2 070	
1943	0,321	2,0/1	1,820	982	563	286	150	138 12		151	243	3 173	
1944	/61	2,094	2,525	546	406	274	142	128	127	161	945	1,660	
1945	989	4,110	2,346	990	457	251	141	137	171	414	1.319	7,506	
1940	2,990	1,335	1,054	783	332	179	115	112	117	142	486	806	
1947	298	1,819	2,441	882	210	171	70.5	82.8	121	327	346	357	
1948	1,479	/16	1,980	4,201	1,232	412	136	138	167	182	221	692	
1949	/35	2,195	6,133	764	358	100	84.5	114	108	127	193	266	
1950	2,352	3,368	1,500	983	491	143	109	98.4	151	563	2.050	5 2 6 9	
1951	5,100	3,778	1,796	636	766	210	121	115	126	207	682	6.422	
1952	7,670	4,345	3,433	954	548	325	201	243	202	196	199	5217	
1955	8,900	937	1,926	1,068	797	482	258	252	240	279	784	732	
1954	4,945	3,551	2,720	2,538	657	224	152	189	278	311	770	2 0 3 9	
1955	1,794	876	650	1,176	718	243	165	150	177	226	306	8 945	
1950	9,712	7,121	1,860	706	548	214	148	144	174	269	304	184	
1957	1,190	3,500	3,080	1,070	1,456	492	195	139	242	1,605	804	2.021	
1950	4,335	14,610	4,668	5,487	655	403	215	175	196	246	247	141	
1959	2,000	4,378	866	448	182	143	175	179	202	276	279	231	
1961	1 226	4,755	3,177	1,081	55.7	232	194	181	228	238	496	2,279	
1962	740	3,012	2,864	1,009	566	326	300	316	347	286	535	1.184	
1963	1 711	0,595	3,642	616	249	151	163	185	. 183	1,369	576	2.046	
1964	2,810	4,529	2,230	4,841	1,167	359	196	17.2	211	309	1,834	574	
1965	6,662	1 226	580	333	161	161	197	178	175	176	1,028	8.712	
1966	5 457	2 2 2 2 2 2	546	2,328	601	200	178	215	220	269	1.288	1.629	
1967	5 232	3,323	1,590	651	355	157	185	204	205	242	1,612	3.607	
1968	3.574	2,302	3,290	3,603	982	518	252	259	311	352	314	951	
1969	9 207	7.240	2,484	493	180	172	205	229	182	198	272	4,113	
1970	13 660	3 200	2,118	939	531	304	241	243	205	242	320	4,142	
1971	4 802	752	1,191	388	166	_202	233	256	181	175	1,086	6.370	
1972	1 160	1 107	2,284	1,150	533	302	205	228	208	214	199	917	
1973	8 860	60 6 206 0 707 68		686		211 205		227	232	325	1,236	1.927	
	0,000	0,300	2,121	/52	347	232	258	249	205	367	5,293	4.865	

YEAR					Month	y mean	stream	flow in	ft <sup>3</sup> /s			
	Jan	Feb	Mar	Ар	Ma	y Jur	n Ju	I Au	a Ser		No	
											1 10	/ Dec
1974	7,68	31 2,11	0 6,95	53 4,09	6	28 2	55 2	19 3	31 36	30 36	30 2	70 607
1975	86	6 7,22	4 6,77	3 1,25	59 6	05 20	58 20	07 2	25 26	35 34	16 30	12 420
1976	25	5 46	3 74	3 56	50 1	73 20	03 20	26 20	20 18	33 16	2 17	70 420
1977	90	9 58.	7 14	6 55	.7 85	.1 81	.3 80	.5 8	35 67	4 33	7 41	1 124
1978	11,95	0 6,13	9_3,99	4 2,14	0 72	21 32	27 2'	18 22	23 20	)5 17	0 26	1 225
1979	1,70	1 4,24	1 2,35	5 79	6 64	15 18	30 22	22 20	8 18	9 31	3 1 50	5 2 7 20
1980	6,48	4 6,71	4 2,41	4 84	3 46	67 23	37 20	07 21	8 19	7 22		9 2,750
1981	2,84	3 2,03	4 1,69	4 60	5 24	15 15	9 23	34 21	6 19	2 25	2 3 82	0 909
1982	4,98	0 4,43	6 3,50	9 6,59	2 74	1 36	6 20	8 26	0 21	5 22	3 1 65	4 0,040
1983	6,22	7 8,27	0 11,81	0 3,48	4 1,63	8 44	1 28	4 24	4 25	4 23	9 3.80	0 8 044
1984	2,01	2 2,16	0 1,49	2 76	4 28	8 11	2 21	6 19	1 15	3 18	1 2.40	2 1 2 2 2
1985	64	6 1,79	2 1,34	5 69	9 22	1 22	6 21	7 19	3 20	0 18	3 35	2 1,203
1986	2,60	/ 14,650	4,899	9 66	7 33	8 24	3 24	6 23	9 24	5 18	3 16	7 204
1987	5/0	1,519	2,607	44:	3 22	8 14	3 16	1 16	2 14	7 17	10	1 2 1 18
1988	4,07	3 887	303	3 265	5 29	7 10	3 89.	2 93.	2 10	3 11	2 306	501
1989	1,00	3 362	4,799	1,049	36	2 18	7 17	5 19	9 19:	2 316	224	204
1990	1,408	1,553	669	182	2 60	4 43	5 16	0 160	144	1 210	274	
1991	93.8	144	5,302	863	348	B 139	39 139 151 146 150		122	203		
1992	561	4,088	1,967	649	30	5 186	S 192	2 18	1 172	190	173	203
1993	8,686	5,168	1,670	1,181	707	7 672	2 23	5 194	1 171	348	232	857
1994	987	2,746	582	275	193	193 115 93.4 121 159		103	237	735		
1995	14,490	2,205	10,150	1,826	1,480	) 382	2 229	204	190	168	172	2 237
1990	5,032	1,046	3,426	1,337	939	364	205	174	171	225	393	6.471
1009	10,270	1,637	797	419	277	178	158	160	179	181	967	1 531
1990	1,520	10,450	2,978	2,332	1,595	972	259	205	265	249	807	1,348
2000	1,529	7.221	3,807	2,054	519	261	210	232	199	· 177	336	388
2000	787	2,279	3,1/3	807	431	238	194	207	193	206	195	257
2002	1 402	3,330	1,918	270	178	108	94	102	159	135	1.052	5.057
2003	4 050	1,540	926	520	286	186	173	154	157	169	181	7.099
Average (cfc)	4070	1,000	1,000	3,071	2,080	393	281	211	185	ND	ND	ND
Average (MCD)	4076	4050	2770	1450	577.5	269.1	186.6	185.1	190.4	268.1	762	2500
5% of Average	2034	2618	1790	937.2	373.2	· 173.9	120.6	.119.6	123	173.3	492.4	1615
	121 70	120.00									,,,,,	
1% of Average	131.70	130.88	89.52	46.86	18.66	8.70	6.03	5.98	6.15	8.66	24.62	80.77
(MGD)	26.24	26 10	17.00									
	20.04	20.10	17.90	9.37	3.73	1.74	1.21	1.20	1.23	1.73	4.92	16.15

ND: No Data Note: ND readings are not included in calculating average flows.

# **APPENDIX B**

Design Drawings for the Dry Creek WWTP Sequencing Batch Reactor – Phase II Expansion (bound separately)

# APPENDIX C

Russian River, Stream P1, and Stream A1 Receiving Water Quality Data

## Stream A1 and P1 General Water Quality Data Parameters (10/20/04)

ANALYTE	Method	Reporting Limit	Stream A1	Stream P1
General Water Chemistry				· ·
рН	EPA 150.1	2.00	7.14	7.62
TDS (mg/L)	EPA 160.1	10	280	270
TSS (mg/L)	EPA 160.2	_ 20	210	50
Hardness (mg CaCO <sub>3</sub> /L)	SM 2340B	4.6	310	190
Ammonia (mg-N/L)	EPA 350.3	0.10	0.20	0.085
TKN (mg/L)	EPA 351.2	0.50	0.66	0.26
Orthophosphate (mg-P/L)	EPA 365.3	0.05	ND	0.15
Total Coliform (MPN/100 mL)	SM 9221	2	1600	1600
Fecal Coliform (MPN/100mL)	SM 9221	2	1600	500
Oil and Grease (mg/L)	EPA 413.1	5	ND	. ND

Notes

ND = Not Detected

NA = Not Available

Stream A1 was sampled ~20-ft east of Hwy 128 on 10/20/04.

Stream P1 was sampled ~200-ft east of Hwy 128 on 10/20/04.

## Stream A1 and P1 Water Quality Data Parameters (10/20/04)

	1.										_		_							
	CTR.	Analyte *	Method	Reportin	Stream A	tream P		Analyte *	Mei	hod	thor ting	IA mart	1d wea	E ANALYTE	, Mett	hod	orting Mit	¥.	T E	7
	Meta	is (ugit)				+-	+	with the line of t			ľ č	5	5	U			13		1 3	1
		Aluminum	EPA 200	a 1000 n	0 20000	1.000	. 1.	mi-Volable Organics (ug/L)					Т	Organochloring Pesticides	(med )		<u> </u>	~		4
	1	Antunday	EPA 200	8 5.00	3.78	1	1	2 J Z-Uspnenyinydrazine	EPA	8270		1	1	102 Aldran	504				1	E
1	2	Arsenic	EPA 206	2 5.00	33	1.2	1	5 2,4.6- (noniarophenal	EPA .	8270	i i	1	1	103 alpha-BHC	574 C				1	
- 1		Banum	EPA 200	8 10.00	310	150	Ľ	2.4-Dichlorophanol	EPA	8270	1		ł	104 beta-BHC	504 0					Ľ
1	3	6 eryaum	EPA 200	8 1.00	0 55	0.092		2.4-Dimenygnenol	EPA	\$270				105 delta-BHC	FPAA				1	1
	4	Caomum	EFA-200	8 0.50	1 1	0.24	4	2 fuDintrochanal	EPAI	3270		1		Lindane	EPA A	041				
	5	Chromuth VI	EPA 7196	~			1 2	2 5-Dinitratoiuene	EPA	3270				107 Chlordane	EPA #	041				
	56	Chomum	EPA 200.	8 10.00	71	25	1 71	2-Chloronaothaise	EPA 8	270				110 4.4-000	EPA B	081				E
	-	Hexavalent Chromium	EPA 7199	0.50	ND	3.5			EPA 8	270	·			109 4,4-DDE	EPA &	081	- 1			
1	5	Copper	EPA 200	10.00	110	13	45	2-Chlorophenol	<b>5</b> 0		i						- 1		1	1
1		Man .	EPA 200 (	300.00	1 0 x 10*	8 5700	48	2-Methyl-4.5-Osnatrophenol	604 47	200				108 4.4-DDT	EPA 80	180				
1	1	Lead	EPA-200 8	3 00	86	13	50	2-Narophenol	504 4		. 1			111 Dielann	EPA 80	180		j		
1		wanganese	EPA 200.0	30.00	410	89.	78	3.3-Dichlorobenzigne	504.5					112 Endosulfan I (alpha-End	osullan) EPA 80	181	1			i i
E		wercury (ngrt)	, EPA 245.1	0.20	0.035	ND	69	4-Bromophenyi-phenylether	EDA B		. 1			113 Endosullan II (beta-endos	sulfan) EPA 60	181	- 1		. 1	1
1	, jo e	VK.R.01	EPA 200.8	10.00	53	30	52	4-Chloro-3-methylphenoi	EDA S				ŀ	114 Endosullan Sullate	EPA BO	181	- 1	1	- 1	1
1		and and a second se	EPA 200.8	5.00	ND	i ii i	72	4-Chlorophenyl-phenylether	EPA 81	20	.			115 Endag	EPA 60	81	- 1	- 1	- 1	1
	12 1	halium.	EPA 200 8	1 1 00	0.3	ND	51	4-Nerophenol	EPA N	70	- ·[			116 Endrin Aldenyde	EPA 80	81		- 1	·	
1	13 7	inc	- EPA 200 8	1.00	. ND	ND	ł	4.6-Dinttro-2-methylphenol	EPA 87	70		· • ]	.	117 Heplachior	EPA ŞO	81	- F	- 1	- 1	
Þ	olatik	es (ug/L)	EPA 200.8	20 00	160	43	56	Acanaphthene - 2 METHOOS	EPA 6	10 1	490	NO I	ND	t so rteptachioł Epozide	EPA SO	6i	- 1	- 1	1	
ŀ		1.1-Trichloroethane	ED4 stern			1	5	Aconaphinylene	EPAS	10 10	960	ND I	NO	126 Tom/sha	EPA 80	6i   .	. I.	- 1	· 1	
	17 1	1.2.2-Tetrachloroethans	EPA 83600	200	NO	ND	58	Anthracene	EPA 5	10 0	049	ND	ND	ther Pesticide - tone	EPA 60	81		_		
1	12 t	1.2-Trichlorogihane	EPA A2600	0.50	NO	ND	59	Benzidine	EPA 82	70	- 1	- T	-	Alachior		1	T	T	7	
L	1	1.2-Trichloro-1.2.2-Trifluoretha	INE EPA 82608	× 30	NO	ND .	60	Benzo(a)anthracene	EPA 61	0 0	049	ND I	ND	Atrazine	EPA 823	70		1		
	8 1.	1-Dichloroethane	EPA 8260A	1.00	ND		62	Benzo(b)fluoranthene	EPA 61	0 0	049	ND	NO	Carbolucan	EPA 814	14			· 1	
	10 1,	1-Dichloroethene	EPA 82608	0.50	ND .		64	Benzo(k)Ruoranihene	ÉPA 61	0 0	049	ND	ND	Chlorpyrifes (Durstram)	EPA 63	1		ŀ		
1	D1 1,	2.4-Trichlorobenzene	EPA 8260B	5 00	NO	10	61	Benzo(a)pyrene	EPA 61	a  a	049	ND	ND	Diazmon	EPA 814	"			- 1	
1	1.	2-Dibromo-3-chloropropane*	EPA 82608			70	63	Benzo(g,h,i)perviene	EPA 61	0 0.	098	ND .	ND	Molenate	EPA 8141	2	1		- 1	
17	5 1.3	2-Dichlorobenzene	EPA 82608	2.00	ND	ND	65	Bist2-Chloroethoxy)methane	EPA 827	0		ľ	- 1	Oxamyi	EPA DIAL	2	1		Ē	
12	9 1,3	2 Dichloroethane	EPA 8260B	0.50	ND	ND	67	Bristz-Chloroennyi) elher BristZ.chloroennyi	EPA 827	0	- 1	E		Smazine	EPA 2141		1		- 1	
13	1 12	2-Dichloropropane	EPA \$260B	0.50	ND	ND	68	Bis/2 Filtrelberv/Johnsteiner	EPA 827	°			_ L	Thiobencarb	EPA 8141	7	- I			
Ľ		- Unchiorobenzene	EPA 5260B	2 00	ND	ND	70 1	Butylbenzyiphthaiate	EPA 827	2	- I ·		· c	Nonnated Acid Herbicides (u	1/L)	1-	+	-+-		
17	7 14	Ochoropropene	EPA 82608	- 1			73 (	Chrysene	EPA 610	. I.,				2,4-D	EPA 8151	4				
2	5 2.6	This control of the second sec	EPA 82606	2.00	ND	ND	74 (	Dibenzo(a,h)anthracene	EPA 610			<u> </u>	<u>"</u>	Beniazon	EPA 8151	~				
1	Ac	Piero (Pronenal)	EPA 502.2		1		79 (	Nethylphthaiste	EPA 827		130	۳Ľ	°	Dalapon -	EPA \$151	4	1			
1	Acr	ly konitnie	EPA 82606	5.00	ND	NO	30. C	emethylphthajate	EPA 8270		1	1		U+2-othylhexyl)adipate	. EPA 506					
15	Ber	trene	EPA 82508	2.00	NO	ND 8	81 0	h-n-butyiphthalate	EPA 8270		1		1	Dinoseo	EPA 8151/	4				
34	. Bro	momethane (methyl bromate)	EPA 82508	200	20	ND	4 6	r-n-octylphihalate	EPA 8270					Fodothal	EPA 549.2					
,21	Car	tion tetrachionde	EPA 8260B	0.50	ND		16 F	woranihene .	EPA 610	0.0	49 N	0 4	0	Givphosate	EPA 548 1	1				
32	Cne	orobenzene	EPA 82608	2.00	ND			luorene	EPA \$10	0.0	98 N	o ( 🛚	0	Pentachiorophenol	EPA 547					
24	Chk	0/oethane	EPA 82508	2.00	ND	NO	он 0 ц	erachiorobenzone	EPA 8270					Pictoriam	EPA 8151A	ſ	1			
35	Chi	promethane (methyl chloride)	EPA 82608	2.00	0.19	223 9	с п 1 н	exactionocyclopentadiane	EPA 8270					Silver (2.4.5-TP)	EPA AISIA	1			-1	
	Cis-	1 2-Dichloroethene	EPA 8260B	050	ND	ND 9	2 10	deno/1 2 3-cd/muran	EPA 8270			N 1	0 🔽	stiles (úgiL)			4	+	-	
36	- Dich	Noromethane	EPA 82608			9	3 15	Phorone	EPA 610	0.04	19 N	N	P	Acetone	EPA 82608		1	1		
22	Ethy	intenzene	EPA 82608	2.00,	ND	ND 94	4 Na	phalene	EPA 62/0	·		1.		Bromobenzene	EPA 82608		1.	1	1	
89	Her	achiorobutadeses	EPA 8250B	1		95	5 NI	robenzene	EPA 8770	10.49		<b>N</b>	2 I	Bromochloromethane	EPA 8260B		1		1	
	мтв	E	EPA 8260B	1.00	ND	¥D   96	i N-	Nerosodimethylamine	EPA 8270					2-Butanone	EPA 8260B			1	1	
	Napl	thalene	EPA AZEOR		ND I	40 97	N-	Nilroso-di-n-propylamine	EPA 6270	1	1	1	1	n-olybenzene	EPA 8250B		1	1	1	
	Styre	me	EPA azenal		NO 11	40   98	.N-	Nitosodiphenylamise	EPA 8270	1	1		1	Sec-Dutylbenzene	EPA 82608		t	1.		
38	Tetra	chloroethene	EPA 82508	1.00		4D   53	Pe	ntachiorophenol	EPA 8270	1		1		Cardon and C	EPA 82608			Ł		
39	Talue	nne ,	EPA 82508	2.00	013	99	Ph	enanthrene	EPA 610	0.045	ND	NC		2-Chlorotoiver-	EPA 82608	1		1	1	
40	irans	1,2-Dichloroethene	EPA 62608	00	ND	in 1	Phi	800	EPA 5270	1	1.	1	1	4-Chlorotokuaria	EPA 82608			1	1	
43	Trich	broathene	EPA 82608	2.00	ND N	0	Br	2.athriberthat	EPA 610	0.049	ND	ND	1	1.2-Dibromoethane (EDB)	EPA AJEAN			1	1	
	Inch	brofluoromethane	EPA 8260B	.00		O PCB	د سا د'ا	a/L)	EPA 8270	L	<u> </u>			Dibromomethane	EPA AZENO		t i	1	1	
	Villy) Yuda -	chionde .	EPA 8260B 0	50	ND N	0	Aro	 chior 1016	6 Da tar-		1	<b>—</b>	1	Ochlorodifluoromethane	EPA 82608	·		1	1 .	
	Тана	e ( folal)	EPA 82608				Aro	chior 1221	EPA 8082			Į		2,2+Dichloropropane	EPA 82508	1		1	1	
27 27	Brom	normalinenes (ug/L)				7	Aro	chior 1232	5PA 4042		1	1	1	1,1-Dichioropropane	EPA 82608			í	1.	
20	Brom	non-monemane [	EPA 8260B 0	.50	ND N		Aro	thior 1242	EPA AGE2			1		1.3-Dichloropropane	EPA 8250B	- 1				
26	Chion	of Gran (Init biogramethane)	PA 82608 2	00	ND N	2	Aroo	thior 1248	EPA 8047			Į. –		cis-1.3-Dichloropropene	EPA 82608	0.50	ND	ND	1	
23	Dibror	nochioromethane	PA 826082 0	50	ND N		Aroc	hlor 1254	EPA 8087					trans-1.3-Dichloropropene	EPA 8260B	0 50	ND	ND		
	_			30 1 1		2	Aroc	hlor 1260	EPA 8082		1	1	1 -	rieon 113	EPA 82608 1	0 00	ND	ND	ľ	
						Othe	r Pric	rity Pollutents					1	A TITL ANODA	EPA 82608	. 1		-	1	
						1.2	Asbe	15105 (MF/L:>10 um)	EPA 600				1	P-ISOPTOPYlickup	EPA 82608		1		1	
		· · · · ·				1	Dia-	nyan (ug/L)	GC-FPD				1	Methylene chiorde	EPA 82608				1	
						1 .	Món	butythe (ug/L)	GC-FPD	Í				Melhyl-2-pentanone	EPA 82608 2	00	ND I	DN	1	
						14	Cyan	rde (ug/L)	GC-FPD					Propylbenzene	EP/C 82600	۰F		.	i i	
les'						16	Dear	nas (pg/L) '	EPA 1512	005	ND	ND		1,1,1,2-Tetrachloroethane	EPA 82608	. 1	- 1		1	
alife	ina 1	and Rule (CTR) constrained	antifaction in the						2.0 1913					2.3-Trichlorobenzane	EPA 82506	1			l.	
lesu	its /ep	ionad as micrograms per liter -	Interst other	iber				1	• •		Ĩ			2,3-Trichloropropane	EPA 82608	1	1	. 1		
olaj	Dore	concentration reported as equ	walent TCDO -	noned										2.4. Tranelbyibenzene	EPA \$260B		1			
Na m	Al w	as sampled -20-it east of Hwy	178 on 10/20/04		in in picogra	ins per l	iter							www.acetate	EPA 8260B		1			
ham	P1 w	as sampled -200-A east of Hwy	128 on 10/20/0	н								1	,	.p-Xylene	EPA 82608	. 1	1	- 1	•	
												1		Xylane .	EPA 826081 0	201		ND		
		•										-			a stand 0	~ [ '	w [	ND		

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### Russian River County Sanitation District Receiving Water Data\*

					1				1
DATE	RIVER FLOW	. HA	RDNESS	UPSTREAM	PH	TU	REIDITY	DISSOL	ED OXYGEN
DATE	(mad)	mail as CaCD3	DOWNSTREAM	UPSIREAM	DOWNSTREAM	UPSTREAM	DOWNSTREAM	UPSTREAM	DOWNSTREAM
9-Jan-96	596	140	120	79	7.8	7.8	92	- mg/i	mg/i
5-Feb-96	31008	76	74	7.6	7.6	146	150	8	8.1
13-Mar-96	6466	· 70	72	7.8	7 8	74	80	9.8	0.2
• 3-Apr-96	2739	84	82	7.8	7.8	51	51	9.4	**
1-May-96	634	110	110	8.1	8.1	8.8	6	86	91
2-Oct-96	110	110	100	8	8	7	5	8.8	86
6-Nov-96	393	110	98	8.3 ·	8.2	3	3	10.8	10.3
4-Dec-96	327	110	110	8.1	. 8.1	2.8 .	• 34	10.5	5.2
8-Jan-97	8731	88	88	77	7.7	182	181	101	10.5
12-Feb-97	1099	+120	110	8	8	25.6	26	9.6	9.5
4-Mar-97	502	.120	410	83	83	11	121	11.7	12.1
2-Apr-97	259	110	110	81	8.2	5.7	5.8	10.3	13.9
7-May-97	253	110	110	8.1	83	4.5	4.9	8.7	7.3
.1-Oct-97	149	98	100	77	7.7	5.9	3.8	8.1	7.7
5-Nov-97	216	- 94	94	7.2	7.8	4.6	4.4	9.2	7.2
10-Dec-97	- 2011	88	86	7.8 .	76	69	66	10.6	10,1
7-Jan-98	3294	93	87	7.3	. 7.9	91.8 -	93.8	11.3	11.2
+Mar-98	3295	99	93	7.8	74	97.2	. 79 8	10.3	11.8
1-Apr-98	2/95	- 96	94	7.7	7.8	125	58.3	10.4	9
0-Jan-99	1228	110 ·	114	8 8	8.1	13.9	3.6		
3-FED-99	1284	96	98	8.1	79	28 7	29.4		
5.51av.00	2121	54	87	1.7	7.7	53.3	46.4		
11.lan-00	767		121 . DV	8/	8.4	5.8	6.1		
9-Eeb-00	1733	102		7.8	8	28.5	19.7		
I-Mar-00	9568	95	. 1(4)	7.2.	7.8		21		······
5-Apr-00	1500	126	170	91	/ 6	85.9	934		~~~~·
3-May-00	428	137	130	81	-0.1 U 7	3	3		
25-Oct-00	109	106	108	81	7.0	3.9.	3.6		
8-Nov-00	165	112 .	118	76	79		2.1		
13-Dec-00	205	108	110						
3-Jan-01		124	122	81	187	18			
7-Feb-01		124	126	8	8	8.4	6.1	·	
7-Mar-01	4472 . 1	88	88	7.6	76	59	58		
4-Apr-01	343 -	150	138	8.7	83	19	25 .		
2-May-01	259	136	134	8.5	8.3	1.7	1.9		
18-Oct-01		104	104	. 83	82	1.2	14		
8-Nov-01	78	108	108	7.8	8.1	2.4	2.4	11.4	11.4
5-Dec-01	3384	78	78	7.5	7.3	70.6	42.4	1	
2-Jan-02	32270			7.59	7.54	228	270	9.25	9.38
3-Jan-02	38470			7 83	7.81	227	264	9.06	9 46
9-Jan-02	9899	112	100	7.5	7.7	56.5	53.6	9.3	86
0-1 eb-02	742	124	120		7.9	5.2	5.1	10,8	10.2
3 hp= 02	174	118	116	79	8	5,4	56	12.6	11.2
3-Apr-02	823 137	116	118	8	8.1	5.7	5	8.8	8.7
6-Nov-07	102	1/0	180	87	82	2.5	4.1	9.7	8.9
1 Der-07	131		109		/8	, 5.5	47	9.3	8.5
8 140-03	2.188	105	118	1.8	8.1	1.1	1.1	9.6	9.1
5-Feb-03	1021	17.1	178	7.0	7.6	40.9	41.1	9.7	8.6
5-Mar-03	763	116	110	81	1.9	- 10.1	12.6		11.8
2-Apr-03	565	150	156			10	8.5		11.2
14-May-03	1178	128	130	7.6	75	1.1.9	177	10.7	13 .
12-Nov-03		122	124	79	8	29		91	9.7
10-Dec-03	3623	104	100	7.7	7.8	25.8	2.3	15.7	154
15-Jan-04	2606	112	120	7.5	71	37.5	19.8	14.9	12.9
4-Feb-04	6728	73	66	7.8	78	97.7	98.6	87	13.8
3-Mar-04	5348	-86	82	7.7	7.7	40.7	42.2	97	96
7-Apr-04	527	126	126	8.3	8.3	3	3	12.8	111
4-May-04	253	122	124	8.1	8.3	3.2	2.4	8.4	86
Average	3601.8	109.7	108.1	7.9	7.9	36.3	35.4	10.3	-9.9
intes		l							
Data was obtaine	tion the RWOCB	on 11 19 04 via e-	mail						
Hank cells indical	c no data was availa	bic							
pstream refers to	the sampling locate	on at Vacation Rea	th approximately 1.000	-It upstream of the	wastewater treatment p	Hant			
www.stream.refere	is to the sampling in-	carton adjacent to I	he Northwood Golf Clu	b approximately 3	00-ft downstream of the	point of discharge			

Town of Windsor Wastewater Treatment, Reclamation, and Disposal Facility Russian River Data

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KUSNAN KIVET	PATA-			_				-												•
	ANMONL	A NITROGEN	UNIONIZEL	VINOMMA C	NITRATE	NITROCEN	A DECEMBER OF								-	-			-	
DATE	UPSTREAM	DOWNSTREAM	UPSTREAM	DOWNSTREAM	UPSTRFAM	DOWNERPEAM	The second	11XOLEN	TOTAL PH	OSPHATE 0	108		NONFILTERAB	<b>JUERESIDUE</b>	DISSOLVEL	OXYGEN	ILVDBOG	EN LON		
	mg/L	nu/L	mu/l.	Purit			ULAREAM	DUWNSTREAM	UPSTREAM	DOWNSTREAM	UPSTREAM	DOWNSTREAM	UPSTREAM L	DOWNSTREAM	UPSTREAM DA	WNSTRFAM!	O M PATOR		LENVEL	ALUKE
	derg	with	rabrahadhan				mwl.	mø/L	mư/L	mg/L	mø/L	mg/L	mø/L	mo/5.					POIKEAM D	OWNST REA
Nov.61					ET AD	48.12	ding -	grub	grub.	grub	ersh	47.6	4					<b>n</b> µ(.	с U	U
			10.1	140		9	=	* ~	1.6					gran	darig .	gruh	da 18	(larig	ta da	data
11-3-4 L			10.0	100	ž	*					4.0	9.4	- =	11.7	8.3	6.8	7.1%	12	·	
Jun-112			80	10.0 -	0.5	6 11				91	27	2 %	27.5	30.8	9.6	. 66	× 2 7			
1 ch-112	0	÷=	100	100	11.61		-	+		4	- 52	2.0	56.0	4.0		C 81				
Mar-up		2.0	101	1014			61.15	0.75	26-12	11 K7	2.7	2.8	20	2.05						
Nov-02	50	0.2	0.01	100				=	1.7	15	3.7	51	24.3	25.0	- 11			/ 1/1	=	5
Decell	0.2		14.01			7	114	14	y	1.7	44	1	110		+			677	4	~
511.112			1.5.1.			~ -	0.75	0.75	1.4	*						2.4	672	6.74	ž	15
			101	10.0 -	7	10	14	0.4	-				40 /	31.7	9.6	6.6	6.96	10.4	-	=
VII-UA1		= =	100 -	10.0	0.78	×	4					- 25	23.0	513	11.4	. 11.	6.99	4.09	1	
Mar-U.	C 11	50	10.0	10.0	-							2.4	R	21.5	11	10.6		31.5		2
Apc.01	0.2	7 O -	. 100	100	0.0			+	15	16	. 24	24	22.5	24.5	6.6					4
Dec-01	6		. 10.0	0.01	0.0		9.0	0.75	0.1	1.0	25	54	30.3	37.0	10 4				•	9
140-08	2	5	0.00	0.00				114	91	16	5 6	3.7	110	37.8	17			<i>G</i> ,	<u>e</u>	5
1-ch-rtd			-				F10 -			17	11	1.6	18.5	112	11			0.81		2
Mar-6H	=	51	AMI								5.1	14	14.1	1 2 2 2			111	507	12	12,
					¥ 11		- 0.4	- 0.4	1.5	1.5	24	22	5.61			471	¥64	6.69	12.2	1.1
Average	0.2	2.0	<0.01	100													101	101	¥	i le
Notes							1.5	1.0	1.6	91	3.1	29 .	P HZ	24.0						
· Pata was received f.	TOTAL RWCK'R	hy mult on December	1961																171	13.8
Hank cells moleate n	to data was availad	le															-			
Destream releas to the	e sampleng locateo.	n at Trenton-Heaklah	nure Burdae					-					ŀ			-	-			
I bow ustream rèlers tu	Control in the second	on muchated days	and the second							-					+				_	
				UN DISEA III III III IIII	es writte structures	de samplang If stream	n Hows do not allo	w for safe sampling.	sumpting shall be e-	sonducted at the Wol	bler Road Bridge th	al economic the Marth	Water of the second sec							